

Memorandum

To: Planning Commission Members

From: Elizabeth J Corwin, PE, AIPC; Planning Director

Date: July 11, 2024

Re: School Bell Child Care Center Expansion

4501 W Highland PIN 11-30-101-002 URSA 24-03/SPR 24-04

The project submitted for your consideration is the addition of a second, modern building to house the child care center at 4501 W. Highland. The center is currently housed in a historic building, but its use as a child care center was established in the 1980's.

The owner has explained that the project is proposed to provide upgraded facilities for the existing clients, not in an effort to expand the service or add new employees. The existing historic structure will be maintained on site and put to a different use, such as office or storage. The Planning Commission may wish to consider what ancillary uses would be appropriate given that this is a residential neighborhood with LV, Lakes and Villages Zoning.

Staff has not received any phone calls or correspondence from the adjoining neighborhood during the public notice period.

Carlisle-Wortman Associates has provided a review and analysis of the site as well as the Special Land Use Approval Standards. Hubbell, Roth & Clark has provided a review of the engineering concepts. The fire Marshall has also provided a report.





117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

Date: July 10, 2024

Special Land Use Review For Highland Township, Michigan

Applicant: School in the Pines Inc. and Marc Kasabasic

Project Name: School Bell Child Care

Plan Date: May 31, 2024

Location: 4501 West Highland Road

Parcel #11-30-101-002

East of Highland Road and South Tipsico Lake Road Intersection

Zoning: Lakes and Villages (LV)

Action Requested: Special Land Use Preliminary Site Plan Approval

PROJECT AND SITE DESCRIPTION

The applicant has submitted a preliminary site plan dated May 31, 2024, that proposes an expansion of the existing childcare center at 4501 West Highland Road (#11-30-101-002) with an additional building is 7,621 square feet. The number of students at the daycare center is proposed to increase from fifty (50) students to ninety-six (96) students. The existing building is proposed to remain and continue to be used. A new dumpster pad, concrete walk, and an asphalt parking area.

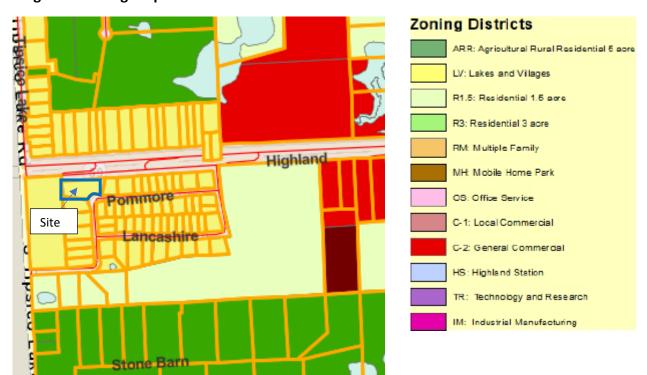
The proposed building is proposed on the east side of the parcel across from the existing building and parking area. The proposed Child Care Center is permitted through Special Land Use approval in the Lakes and Villages Zoning District. Figure 1 provides an aerial image of the current site outlined in blue. Figure 2 shows the zoning of the site and surrounding parcels.

Figure 1. Aerial Image of Subject Site and Vicinity



Source: NearMap

Figure 2. Zoning Map



The intent of the Lakes and Villages Residential District is:

"It is the intent of these districts to provide for single-family detached residential development in Highland Township at varying densities which are consistent with the

existing and desirable future pattern of development in the Township. It is the goal of the Township to protect single-family residential development from the encroachment of incompatible land uses. The Residential R-3, R-1.5 and LV Zoning Districts are distinguished by varying residential densities to accommodate a wide range of existing and future residential preferences in the Township.

The R-3 and R-1.5 Districts are intended for lower density rural residential development. The R-3 and R-1.5 Districts are designated in the northern, western and southern portions of the Township, consistent with the Township Master Plan. Restricting these areas to lower density use will help ensure that the majority of future development occurs in the central portion of the Township, where public services can be more economically provided.

The LV - Lake and Village, Residential Zoning District consists of historical small lot settlements around the lakes as well as developments within the Township. LV regulations are intended to preserve established development patterns. No new LV Districts shall be created in the Township, except that new parcels within the LV Zoning District may be created through land division and combination."

Items to be Addressed: None.

NEIGHBORING ZONING, LAND USE AND MASTER PLAN

Table 1 lists the existing land use, zoning, and master plan future land use designations of the subject site and neighboring properties. Figure 3 shows the land use designations for the subject site and surrounding properties from the Future Land Use Map last updated on September, 9, 2021.

Table 1. Existing Land Use, Zoning Districts, and Future Land Use Designations

	Existing Land Use	Zoning	Future
Subject Site	Day Care Center	Lakes and Villages (LV)	Medium and Small Lot (LV Zoning)
North	Single Family Residential	Lakes and Villages (LV)	Medium and Small Lot (LV Zoning)
South	Single Family Residential	Lakes and Villages (LV)	Medium and Small Lot (LV Zoning)
East	Single Family Residential	Lakes and Villages (LV)	Medium and Small Lot (LV Zoning)
West	Vacant	Lakes and Villages (LV)	Medium and Small Lot (LV Zoning)

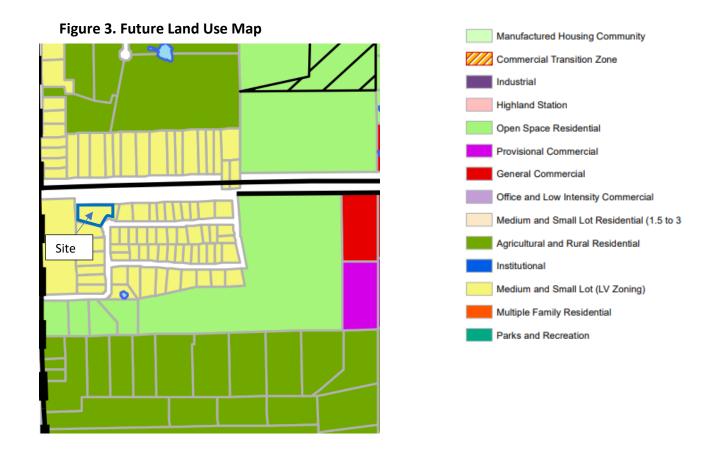
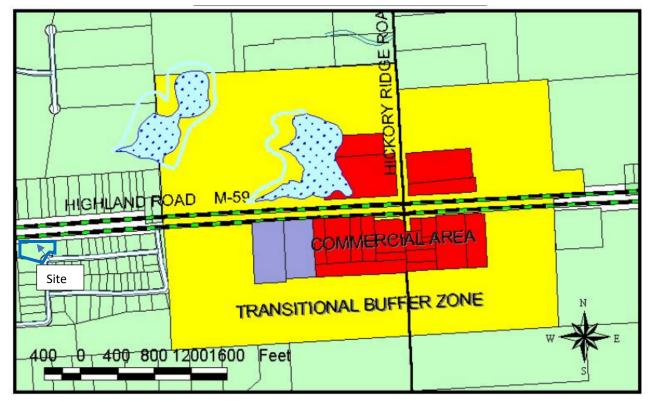


Figure 4. West Highland Micro-Area Analysis Commercial Area and Potential Transitional Buffer



School Bell Child Care - Preliminary Site Plan Review July 10, 2024

The 2000-2020 Comprehensive Land Use Plan does not address day care centers in areas planned for residential specifically. The site is located in the West Highland Micro-Area Analysis Master Land Use Amendment adopted in 2001. In that document, the subject site and the surrounding property are designated as medium and small lot residential. That document emphasizes the retention of "rural atmosphere" by the preservation of open space along major throughfares, such as Highland Road. The micro-area analysis seeks the development of distinct, compact commercial districts, specifically at Highland Road and Hickory Ridge Road, as shown in Figure 4.

Items to be Addressed: None.

AREA, WIDTH, HEIGHT, SETBACKS

The following table summarizes the Coverage, Placement, and Height Regulations for the site plan associated with the use. The proposed structures appear to meet all dimensional regulations of the zoning ordinance.

Table 2. Coverage, Placement, and Height Regulations

	Required	Provided
Lot Coverage	35 % Max	34.7%
Front Setback 40 Feet		40 Feet
Side Setback	10 Feet Minimum (Minimum Total of Both Sides=30 Feet)	10 Feet >30 Feet
Rear Setback	40 Feet	52 Feet
Building Height	2 Stories or 28 Feet	27 Feet 10 Inches

Items to be Addressed: None.

BUILDING LOCATION AND SITE ARRANGEMENT

The facility will be accessible from Highland Road (M-59). The proposed layout allows proper vehicular circulation as shown on Sheet 9. The plan includes the creation of an additional reserve field and septic field on the Southern end of the parcel.

Items to be Addressed: None.

PARKING, LOADING

The required parking for daycare centers, per Section 11.02 of the Highland Township Zoning Ordinance, is two (2) spaces, plus one (1) additional space for each eight (8) children of licensed authorized capacity. The proposed building will be licensed for ninety-six (96) children with the requirement of fourteen (14) parking spaces. The applicant has provided fifteen (15) parking spaces including one (1) handicap accessible space.

Items to be Addressed: None.

FENCING

The applicant is proposing 778 lineal feet of vinyl fencing at heights of four (4) feet and six (6) feet along the rear perimeter. The plans do not delineate where the fence is proposed to be four (4) feet or six (6) feet.

Items to be Addressed: 1) Applicant to show where fence is proposed to be four (4) feet and where fence is to be six (6) feet. 2) Planning Commission input on screening requirements per Section 12.04.

SITE ACCESS AND CIRCULATION

The applicant provides an emergency vehicle circulation plan on Sheet 9, which demonstrates proper vehicular traffic flow.

We defer to Engineering and the Fire Chief for further consideration.

Items to be Addressed: None.

NATURAL FEATURES

The parcel is mostly developed. The remaining eastern half of the site is moderately wooded with a stand of trees.

The site slopes from the center outwardly at 1-3% in all directions. The site generally drains from the center to the north onto W Highland Road (M-59) with the remaining drainage following the site slopes.

The NRCS soils on the whole property are Fox Sandy Loam at 1-6% slope which is consistent with the general character of the slopes on the site.

Items to be Addressed: None.

LANDSCAPING

The applicant has provided a landscape plan which meets the Zoning Ordinance requirements with approval and determinations by the Planning Commission, as shown in the Table 3.

Table 3. Landscaping Requirements

Landscaping Requirement	Redilired Provided		Complies	
Screening Between	6 ft height visual screen (Section	6 new conifer trees, 1 new deciduous tree, 5 existing deciduous trees (south)	Planning Commission to determine type	
Land Uses	12.04)	3 existing deciduous trees (east)	conifer hedge, solid wall, or decorative fence	
Parking Lot Landscaping	1 tree + 3 shrubs per 8 spaces Section 12.05 (A)	1.75 trees + 5.25 shrubs for 15 spaces (1.75 trees, 5.25 shrubs)	Planning Commission to determine appropriate location	
Landscaping Adjacent to Public Rights- of-Way	Landscaped berm, wall, or sufficient plantings (3 ft height) Section 12.05 (B)	4-foot high-opacity deciduous shrub row along M-59 side of lot (17 shrubs)	Complies	
	20 ft width for non- residential on Highland Road (M- 59)	Highland Road/M-59: 20 feet, 15 deciduous trees (425 linear feet)		
Greenbelts	12 ft width for other non-residential uses	Pommore Drive: 20 feet, 6 deciduous trees (172 linear feet)	Complies	
	1 tree/30 linear feet	1 existing deciduous tree + 14 deciduous trees (Highland Road)		
	of frontage	5 existing deciduous trees + 1 deciduous tree (Pommore Drive)		
Site Landscaping	15% of site area (7,507 sq ft or 0.243 acres) (Section 12.07)	Total Landscaped 16,115 sq ft (0.370 acres) West Side Yard: 7,053 sq ft East Side Yard: 2,943 sq ft Foundation Planting: 914 sq ft	Complies	
		Rear Yard: 5,205 sq ft	-	

Landscaping Requirement	Required	Provided	Complies
Screening of Trash Containers	All sides screened with opaque fence or wall and gate (Section 12.09)	6 ft height walled enclosure with gate included	Complies

Items to be Addressed: 1) Planning Commission to determine type conifer hedge, solid wall, or decorative fence. 2) Planning Commission to determine appropriate location for parking lot landscaping.

LIGHTING

A sufficient lighting plan has been provided on Sheet E-04. A photometric plan indicates that the light levels at property lines are acceptable. Planned lighting is indicated to be included on Sheet E-01, which is not included.

Items to be Addressed: Include Sheet E-01 for fixture locations, specifications, and other lighting system information.

SIGNS

Site plan does include a wall mounted sign on Sheet A-10. The sign includes 10 individual letters, occupies 1.4 square feet per linear foot of building, and 14 square feet of total wall area. The proposed sign is in compliance with the standards set forth in Section 14.03.

Items to be Addressed: Number and size of signs is to be determined by the intended use of the parcel by the Planning Commission.

FLOOR PLAN AND ELEVATIONS

Floor plan and elevation sheets have been included in the site plan on sheets A-2, A-3, A-4, and A-6.

Items to be Addressed: None.

TRASH ENCLOSURE

The applicant has indicated that a garbage receptacle will be placed in the south-central area of the property and is adequately secured.

Items to be Addressed: None.

SPECIAL LAND USE STANDARDS

Section 6.03(H) lists the standards for all special land uses. Prior to making a recommendation to the Township Board, the Planning Commission must make a determination on the special land use based on these standards.

1. All special land uses shall be designed, located, planned and operated so that the public health, safety and welfare will be protected.

CWA COMMENTS: This standard can be met. However, additional details including the hours of operation and the operation plan required by Section 10.09 (see standard 7 in this section of the review) must be provided in order to assess this standard.

2. The Special Land Use will be consistent with the stated intent of the zoning district.

CWA COMMENT: The standard is met if the Planning Commission feels that expansion of the child care center is compatible with the surrounding single-family residential development. The intent of the residential districts in general is to "protect single-family residential development from the encroachment of incompatible land uses." The intent of the LV zoning district in particular is to "preserve established development patterns."

Day care centers are allowed as a special land use in the LV zoning district, with the assumption that these use could be compatible with single-family residential. A limit on the size or intensity of day care centers in not included in the specific regulations for day care centers in Section 10.09.

3. The proposed special land use shall be in general agreement with the Master Plan designation for the area where the use is proposed.

CWA COMMENTS: The standard may be met if the Planning Commission finds that the proposed expansion will maintain the rural character of the Township and is appropriate outside of the commercial node planned at Highland Road and Hickory Ridge Road. Both the adopted Master Plan and West Highland Micro-Area Analysis Master Land Use Amendment offer little guidance on day care centers in particular, with no mention of this land use.

The Master Plan places priority on preserving the rural nature of Highland Township. The existing building has a rural architecture, similar to a large farm house. The proposed building is more formal and modern in nature, with cultured stone and composite shake style siding. We recommend that the architecture of the proposed building, through the use of similar siding and roof styles, match that of the existing building on site.

4. All special land uses shall provide facilities for safe and convenient vehicular and pedestrian traffic, including but not limited to: turning movements, traffic flow, proximity and relationship to intersections, adequacy of sight distances, location and access of off-street parking, and provisions for pedestrian traffic.

CWA COMMENTS: The standard can be met. We defer to Township staff, engineering, and Fire Department for comments. However, we note that the applicant has not provided traffic patterns for pickup and drop off of the clients, as required by Section 10.09(D) for day care centers. The pickup and drop off circulation are relevant to the evaluation of this standard.

5. All special land uses shall be designed, constructed and operated in a manner that prevents detrimental impacts to surrounding properties such as noise, dust, fumes, smoke, air, water, odor, light and/or vibration, etc. The special land use shall be designed, constructed and operated in a manner that does not detract from area aesthetics.

CWA COMMENTS: The standard can be met. The site is designed in a manner that prevents detrimental impacts to surrounding properties, with proper screening and vehicular access limited to Highland Road. In terms of area aesthetics, we reiterate our previous recommendation that the building materials and roof pitches of the proposed building should match those of the existing building on site.

6. The proposed special land use shall not unreasonably burden the capacity of public services and/or facilities.

CWA COMMENTS: We defer to Township staff and engineering.

7. The proposed special land use shall comply with any specific standards set forth in Article 10, Supplemental Use Regulations, that are applicable to the use.

CWA COMMENTS: The application as submitted does not comply with the standards in Section 10.09 - Day Care Centers, adult or child. Table 4 shows how the applicant has met those regulations.

Table 4. Regulations in Section 10.09

Regulation	CWA Comment
An adult or child day center must have frontage on a major thoroughfare.	The regulation is met. The site is on Highland Road.
The proposed use shall not change the essential character of the surrounding residential area and shall not create a nuisance.	The regulation can be met. More information on pick and drop off is needed.
A child care center located in a residential zoning district may not exceed sixteen (16) hours of operation during a 24-hour period and may not operate between the hours of 10 p.m. and 6 a.m.	The hours of operation have not been provided.

Regulation	CWA Comment
The operator must provide an operations plan indicating: The number of clients to be cared for and number of employees. The operations plan must indicate traffic patterns for pickup and drop off of clients, including designation of suitable parking areas. The operation plan is subject to review and approval by the Planning Commission.	An operations plan has not been provided.
Screening between the outdoor play area adjacent residentially zoned and used property shall be provided in accordance with Section 12.04, Screening Between Land Uses.	The regulation is met. The landscape plan shows 778 lineal feet of fencing.
Fencing of portions of the site where it is necessary to ensure the safety and security of the clients shall be provided unless it is determined by the Planning Commission that the applicant has satisfied this condition through other means. A fenced outdoor play area of at least five thousand (5,000) square feet must be provided.	The regulation is met.
Signage must comply with the provisions of Article 14, Signs.	The regulation is met.

Items to be Addressed: 1) Applicant provides the information needed to evaluate compliance with Section 10.09 2) Planning Commission determination on whether the proposal meets the Special Land Use standards.

RECOMMENDATIONS

We recommend the following items be addressed by the applicant before the Planning Commission takes action:

- 1. Provision on Sheet E-01 for fixture locations, specifications, and other lighting system information.
- 2. Show where fence is proposed to be four (4) feet and where fence is to be six (6) feet.
- 3. Provide the information needed to evaluate compliance with Section 10.09, including an operations plan.

The Planning Commission should make the following determinations as part of a recommendation:

1. Planning Commission determination on screening requirements per Section 12.04.

School Bell Child Care - Preliminary Site Plan Review July 10, 2024

- 2. Planning Commission to determine type conifer hedge, solid wall, or decorative fence.
- 3. Planning Commission to determine appropriate location for parking lot landscaping.
- 4. Planning Commission determination on whether the Special Land Use Standards for a Daycare Center have been met.

Respectfully submitted,

CARLISLE/WORTMAN ASSOC., INC. Megan Masson-Minock, AICP

Principal

CARLISLE/WORTMAN ASSOC., INC.

Grayson Moore Community Planner

517-552-9199

www.hrcengr.com

July 10, 2024

Highland Township 205 North John Street Highland, MI 48357

Attn: Ms. Beth Corwin, P.E.

Planning & Development Director

Re: Preliminary Site Plan Review

School Bell Childcare 4501 West Highland Road Sidwell No. 11-30-101-002

Dear Ms. Corwin:

As requested, this office has reviewed the plans for the above-mentioned project as prepared by Boss Engineering (dated May 31, 2024). The proposed improvements include a new building and parking lot on an existing school site. We have the following comments:

Water Supply & Wastewater Disposal

- 1. The plans indicate that the existing well will be used to supply both buildings. This will require the review and approval of the County Health Department. They may not allow 2 buildings to be served by a single well.
- 2. The plans indicate that the existing and a proposed septic tank and field will be used for wastewater disposal of the buildings. This will require the review and approval of the County Health Department.

Storm Water Management

- 1. All proposed storm sewer improvements will need to be designed in accordance with Highland Township Engineering Design Standards.
- A new underground stormwater detention system will be provided to accommodate the proposed building and parking
 lot. This system will outlet to the ditch along Highland Road and will require the review of the Michigan Department of
 Transportation (MDOT). The proposed metal pipe system is not an approved material and will need to be revised to
 be HDPE pipe.

Paving and Grading Improvements

- 1. The proposed paving and grading improvements will need to be designed in accordance with Highland Township Engineering Design Standards.
- 2. A permit from MDOT will be required for the proposed work within the Highland Road right-of-way.
- 3. Site circulation will need to be reviewed and approved by the Township Planner and Fire Department.

Bloomfield Hills Delhi Township Detroit Grand Rapids Howell Jackson Kalamazoo Traverse City Troy



Soil Erosion Control

 The Soil Erosion and sediment control plans will require the review and permitting of the Oakland County Water Resources Commission.

Summary

The items noted above will need to be addressed in future site plan submittals. This office is available to discuss any of these comments with the applicant prior to the site plan submittal. If you have any questions or require any additional information, please contact the undersigned.

Very truly yours,

HUBBELL, ROTH & CLARK, INC.

Michael P. Darga, P.E.

MPD/mpd

pc: Highland Township; R. Hamill

HRC; R. Alix, File

Charter Township of Highland - Fire Department



1600 W. Highland Rd. Highland, MI 48357 (248)887-9050

Re: 4501 W. Highland Rd. Highland, Mi 48380

July 9, 2024

To whom it may concern:

Project Overview: The proposed site plan for a new building located at 4501 W. Highland Rd

I have reviewed a proposed new building site plan at the above location. All current site concerns regarding fire department access have been addressed.

- 1. Building plans shall be submitted and evaluated for use and further requirements.
- 4. A plan set shall be provided to The Department of Licensing & Regulatory Affairs Bureau of Fire Service for review.

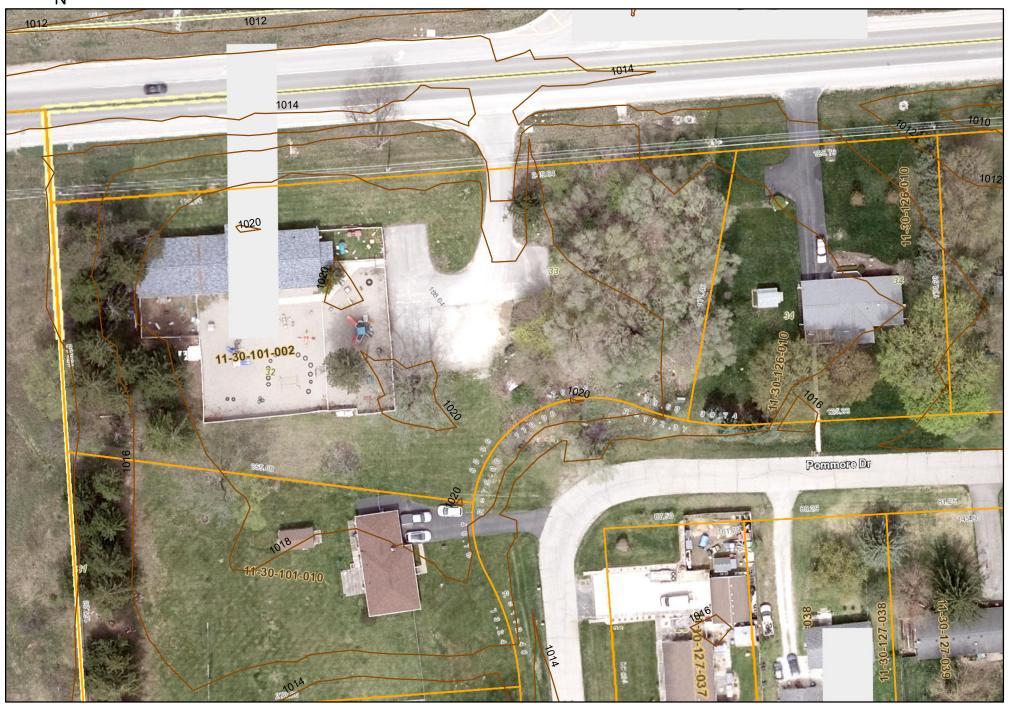
Review and approval by the Authority Having Jurisdiction shall not relieve the applicant of the responsibility of compliance with these codes.

If you have any questions about this plan review report, don't hesitate to get in touch with me.

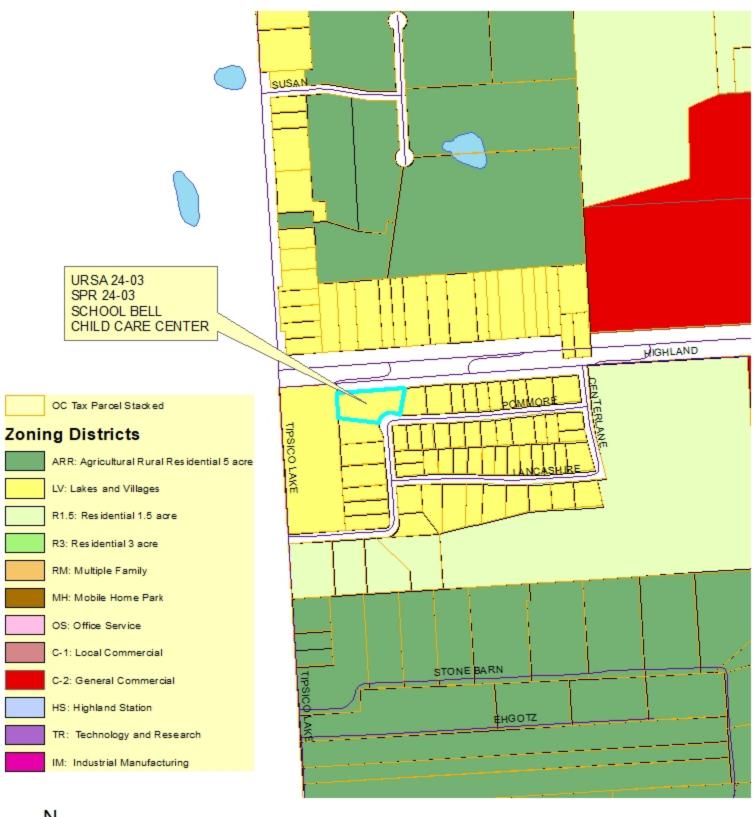
Respectfully submitted,

Shawn Bell Fire Marshal

Highland Township Fire Department



CHARTER TOWNSHIP OF HIGHLAND ZONING MAP





ENACTED: November 18, 2010



Site Plan Review
Rezoning
Use Requiring Special Approval
Land Division
Land Division & Combination
Road Profile
Other

PLAN REVIEW APPLICATION

Highland Township Planning Department, 205 N. John St, Highland, Michigan 48357 (248) 887-3791 Ext. 2

Triginand Township I	danning Department, 200	iv. John St, Highland,	Wilchigan 40337 (240) 667-3731 Ext. 2		
Date Filed:	Fee:	Escrow:	Case Number:		
	NOTI	CE TO APPLICA	ANT AND OWNER		
ARE RESPONSIBLI	APPLICATION, THE AFE FOR ALL APPLICATION OF THE AFE OWNER ALSO AUT	PPLICANT AND OW ION AND CONSULT HORIZES THE TOV	WNER ACKNOWLEDGE ONE OR THE OTHER OR I TANT FEES THAT ARISE OUT OF THE REVIEW O WNSHIP TO PLACE A SIGN ON THE PROPERTY, I MATTER BEING REQUESTED.	F	
	<u> </u>	REQUIRED COPI	ES OF PLANS		
CONSULTAN			PLANS AND .PDF COPY OF PLANS JECT TO CONDITIONS: 5 COPIES AND .PDF COP	Υ	
1. APPLICANT IN	FORMATION	OW	VNER INFORMATION		
NAME:		NA	ME:		
		AD	DRESS:		
			-		
			PHONE:		
EMAIL:	IAIL:				
2. PROPERTY INF	FORMATION				
ADDRESS OR A	ADDRESS OR ADJACENT STREETS:				
			LOT AREA:		
PARCEL IDENT	PARCEL IDENTIFICATION NUMBER(S):				
3. PROJECT INFO	RMATION				
PROJECT NAM	E:				
PRESENT ZON	ING:	PR	PROPOSED ZONING:		
PRESENT USE	:	PR0	OPOSED USE:		
APPLICANT SIGNATURE: Ma			OWNER SIGNATURE: Federica Kasabasic		
			PRINT NAME:		
Public, personally ap signature appears a		ed person whose ed the foregoing	On theday of, before me, a Public, personally appeared the above named person signature appears above, and who executed the for instrument, and he/she acknowledged to me that executed the same.	whose egoing	
State Of Michigan County Of Oakland			State Of Michigan County Of Oakland		
Notary Public:			Notary Public:		

- If there are Co-Applicants and/or Co-Owners associated with this property(ies) to be acted upon, please submit a Notarized Co-Applicant's and/or Co-owner's "Interest in Property Certificate" with this application. The person signing this cover sheet will be considered the official designee for the group and all correspondence will be addressed to this person.
- A notarized letter giving the Applicant authorization to represent the Owner is also permitted in lieu of a signature on this application. The person signing this cover sheet, however, will be considered the official designee for the Owner and all correspondence will be addressed to this person.

HIGHLAND TOWNSHIP IMPACT ASSESSMENT School Bell Childcare

Prepared for:

Property Owner School of the Pines Inc. dba School Bell Childcare Marc Kasabasic

Applicant School of the Pines Inc. dba School Bell Childcare Marc Kasabasic

Prepared by:

Jared Prather, E.I.T.



3121 E. Grand River Howell, MI 48843 517.546.4836 fax 517.548.1670 www.bosseng.com

May 31st, 2024

DISCUSSION ITEMS

Name(s) and address(es) of person(s) responsible for preparation of the General Description and a brief statement of their qualifications.

Prepared by:

Jared Prather, E.I.T. Project Engineer Boss Engineering 3121 E Grand River Howell, MI 48843

Prepared for:

Owner & Applicant:

School of the Pines Inc. dba School Bell Childcare / Marc Kasabasic 4501 W. Highland Road Milford, MI 48380

General Site Description

The project site is on parcel #11-30-101-002 in Section 30, Highland Township, Oakland County, MI 48380.

The subject site is bordered:

- On the west by a large, open parcel 6.10 acres in size zoned Lake & Village (LV).
- On the north by the West Highland Road (M-59) MDOT Right of Way.
- On the east by a single-family residential parcel, 0.75 acres in size, zoned LV.
- On the southeast by Pommore Drive
- And on the south by a single-family residential parcel, 0.85 acres in size, zoned LV.

The subject site is an occupied parcel of land consisting of an operational day care center along West Highland Road with a driveway, paved parking lot, playground, fence separating the driveway from the playground, and wooded area in the east. Sanitary and water services are provided by existing onsite septic and well systems. The existing septic field and reserve septic field is on the southwest side of the playground and the well is on the northern edge of the parking lot. Along the southeastern property bend lies Pommore Drive (See the Existing Conditions Plan). The front of the property is a flat lawn space and the eastern half of the property has a moderately wooded area. There is currently one existing paved commercial drive approach from the north end of the subject property leading to the unstriped parking lot and gravel parking area from W Highland Road (M-59).

Natural Features

The parcel is developed. As described earlier, the property includes a day care building, a paved parking lot, and a playground. The day care center and playground occupy approximately the western half of the site with the paved parking lot in the middle of the site. The remaining eastern half of the site is moderately wooded with a collection of trees.

The site slopes from the center outwardly at 1-3% in all directions. The northern side of the parcel slopes toward W Highland Road (M-59) with slopes increasing from 2% to 6-10%, western side slopes toward the adjacent parcel field from 1-2% to 5-10%, eastern side sloping toward the adjacent property from 2% to 3-5%, and southern side sloping toward Pommore Drive from 2-3% to 10-20%. The site generally drains from the center to the north onto W Highland Road (M-59) with the remaining drainage following the site slopes.

The NRCS soils on the whole property are Fox Sandy Loam at 1-6% slope which is consistent with the general character of the slopes on the site.

Storm Water Management

Storm water will be managed onsite with the creation of a new storm water sewer network. The new school building, parking lot, and some of the modified playground area will be captured in catch basins and directed to the new underground detention system with a restricted discharge to the W Highland Road (M-59) MDOT right-of-way storm ditch and culvert through an overflow control structure. Detailed construction plans will be reviewed by the Township Engineer and the Soil Erosion Control plans will be reviewed and permit issued by the Livingston County Drain Commissioners office prior to construction commencing. Ongoing/periodic soil erosion inspections will occur per County requirements to ensure soil erosion is managed proactively.

Impact on Surrounding Land Use

The proposed special use will conform to existing development patterns and will not negatively impact adjacent properties with added lighting, noise, or air pollution. The proposed project seeks to expand the business operation of School Bell Childcare with the creation of a new schoolhouse building. The site development will comply with Township Ordinances and will meet the current Township, County, and State engineering standards. There will be minimal impact on surrounding land uses or development patterns as there will be no change to these uses. Additional lighting will be designed to illuminate the new schoolhouse building and parking lot and will not impact the adjacent residential homes.

Impact on Public Facilities and Services

The proposed updates to the existing site will not add any additional burden on the fire and police services. Fire trucks will use the M-59 right-of-way to access the site in case of a fire on either side of the site with an additional access behind the site from Pommore Drive. No hydrant coverage exists on the site. Refuse vehicles will have access to the onsite dumpster located at the southern end of the driveway to collect the garbage. The use increases the maximum overall school population to 96 students and 13 staff.

Impact on Public Utilities

Storm water will be detained onsite via the use of an underground detention system. Overall runoff along the northern end of the site toward W Highland Road (M-59) will be reduced below existing conditions. The storm water will be discharged at the restricted rate to the MDOT right-of-way culvert and ditch instead of the existing uncontrolled condition. Detailed construction plans would be reviewed by the Township Engineer and a Soil Erosion Control permit would be reviewed and issued by the Oakland County Drain Commissioner. Water & sanitary services will continue to be provided using the existing well and the existing and proposed septic systems onsite.

Storage or Handling of any Hazardous Materials

No storing or handling of any bulk hazardous materials is expected for this development.

Impact on Traffic and Pedestrians

A Trip Generation summary has been completed using the ITE 10th Edition Volume 2 manual and is included on the plans. The number of students will increase from 50 to 96 and would generate a maximum AM peak total of 75 trips and PM peak total of 76 trips from the arrival to the departure of the site. The entryway will be enlarged to 24 feet wide in conformance with the driveway width requirement along W Highland Road (M-59).

Special Provisions

A special land use permit application will be submitted with the site plan.

Description of all sources:

- Highland Township Zoning Ordinance and engineering standards
- Oakland County Drain Commissioner engineering standards
- NRCS Web Soil Survey
- Institute of Traffic Engineers (ITE) Trip General Manual, 10th Edition

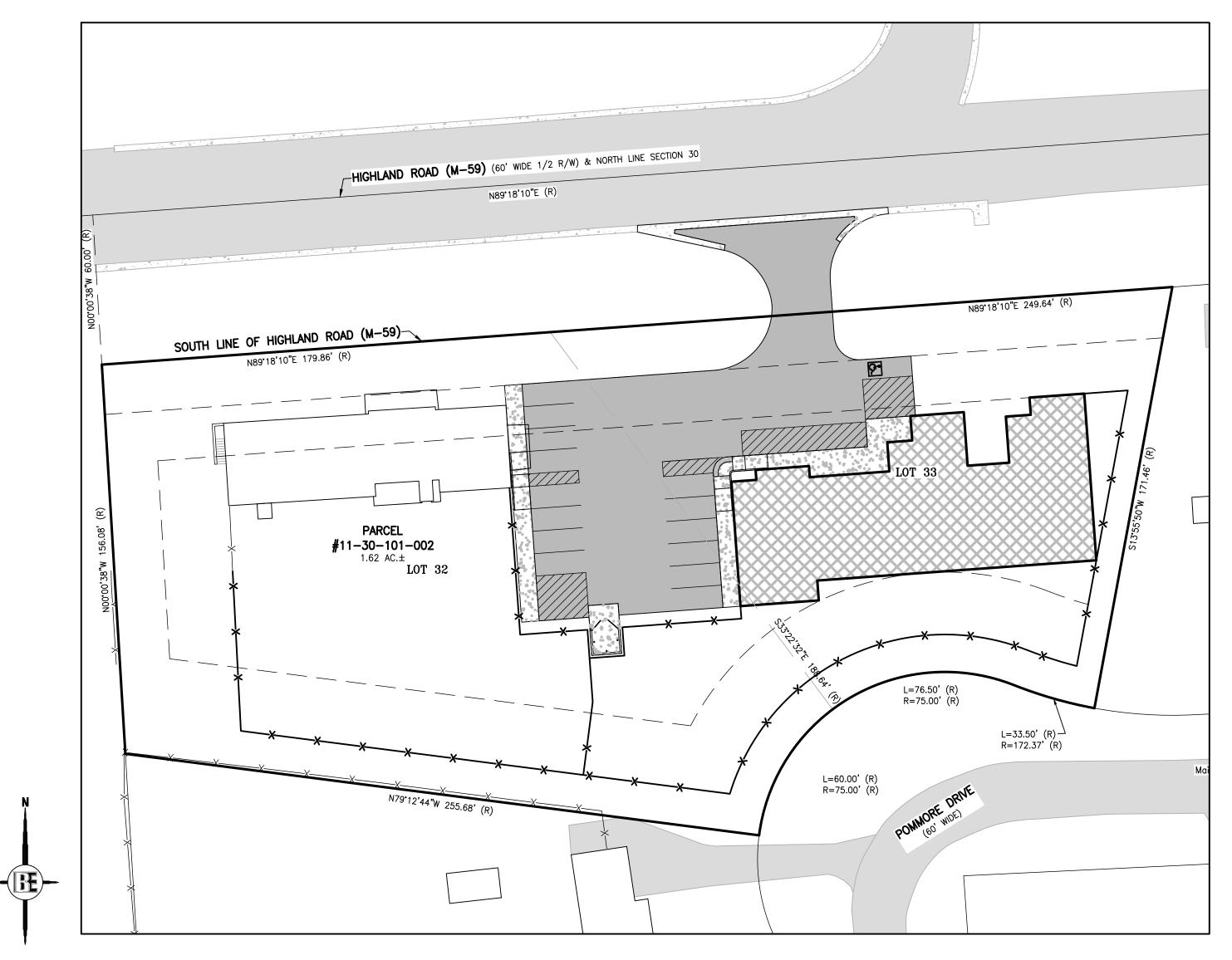
PROPERTY DESCRIPTION:

PROPERTY DESCRIPTION PER OAKLAND COUNTY TAX ROLL:

T3N, R7E, SEC 30 WEST HIGHLAND VILLAGE LOTS 32 & 33

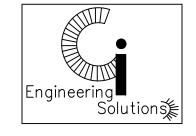
SITE PLAN FOR SCHOOL BELL CHILDCARE ADDITION

PART OF NW QUARTER, SECTION 30 T3N, R7E HIGHLAND CHARTER TOWNSHIP, OAKLAND COUNTY, MICHIGAN 48380



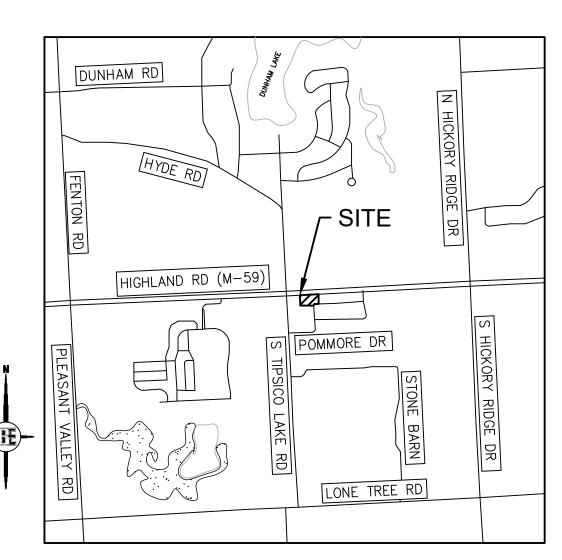
OVERALL SITE MAP

NO SCALE



ARCHITECT:

Ci ENGINEERING SOLUTIONS 15 N WALNUT ST, SUITE 300 MOUNT CLEMENS, MI 480843 CHASE DIES (586)-925-6646 cdies@ciengsolutions.com



LOCATION MAP

NO SCALE

SHEET INDEX				
SHEET NO.	DESCRIPTION			
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15	COVER SHEET GENERAL NOTES & LEGEND EXISTING CONDITIONS & DEMOLITION PLAN NATURAL FEATURES PLAN SITE PLAN GRADING & DRAINAGE PLAN SOIL EROSION SEDIMENTATION CONTROL PLAN UTILITY PLAN VEHICLE CIRCULATION PLAN LANDSCAPE PLAN CONSTRUCTION DETAILS CONSTRUCTION DETAILS HIGHLAND TOWNSHIP STORM DETAILS HIGHLAND TOWNSHIP STORM DETAILS OAKLAND COUNTY DETAILS			
PLANS BY OTHERS CI ENGINEERING SOLUTIONS				
L A	LIGHTING PHOTOMETRIC PLAN ARCHITECTURAL PLANS			

PREPARED FOR:

SCHOOL IN THE PINES INC. dba SCHOOL BELL CHILDCARE 4501 WEST HIGHLAND ROAD MILFORD, MI 48380 MARC KASABASIC 248.830.9642 mkasabasi@ltu.edu

PREPARED BY:



HOWELL, MI. 48843 517.546.4836 FAX 517.548.1670 CONTACT: BRENT LaVANWAY

FOR SITE PLAN APPROVAL ONLY!
NOT FOR CONSTRUCTION

					4
					ISSUE DATE: 05/31/24
NO	BY	CK	REVISION	DATE	JOB NO: 24-048

INDEMNIFICATION STATEMENT

THE CONTRACTOR SHALL HOLD HARMLESS THE DESIGN PROFESSIONAL, MUNICIPALITY, COUNTY, STATE AND ALL OF ITS SUB CONSULTANTS, PUBLIC AND PRIVATE UTILITY COMPANIES, AND LANDOWNERS FOR DAMAGES TO INDIVIDUALS AND PROPERTY, REAL OR OTHERWISE, DUE TO THE OPERATIONS OF THE CONTRACTOR AND/OR THEIR SUBCONTRACTORS.

GENERAL NOTES

- 1. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED TOWNSHIP, COUNTY, AND STATE OF MICHIGAN PERMITS.
- 2. A GRADING PERMIT FOR SOIL EROSION-SEDIMENTATION CONTROL SHALL BE OBTAINED FROM THE GOVERNING AGENCY PRIOR TO THE START OF CONSTRUCTION.
- 3. IF DUST PROBLEM OCCURS DURING CONSTRUCTION, CONTROL WILL BE PROVIDED BY AN APPLICATION OF WATER, EITHER BY SPRINKLER OR TANK TRUCK.
- 4. ALL CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH LOCAL MUNICIPAL STANDARDS AND SPECIFICATIONS.
- 5. PAVED SURFACES. WALKWAYS, SIGNS, LIGHTING AND OTHER STRUCTURES SHALL BE MAINTAINED IN A SAFE,
- 6. ALL BARRIER-FREE FEATURES SHALL BE CONSTRUCTED TO MEET ALL LOCAL, STATE AND A.D.A. REQUIREMENTS. WHERE EXISTING CONDITIONS AND/OR THE REQUIREMENTS OF THE PLANS WILL RESULT IN FINISHED CONDITIONS THAT DO NOT MEET ADA REQUIREMENTS, THE CONTRACTOR SHALL NOTIFY THE DESIGN
- 7. ANY DISCREPANCY IN THIS PLAN AND ACTUAL FIELD CONDITIONS SHALL BE REPORTED TO THE DESIGN ENGINEER PRIOR TO THE START OF CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL SETBACKS, EASEMENTS AND DIMENSIONS SHOWN HEREON PRIOR TO BEGINNING CONSTRUCTION.
- 8. THE CONTRACTOR SHALL CONTACT ALL OWNERS OF EASEMENTS, UTILITIES AND RIGHT-OF-WAY, PUBLIC OR PRIVATE, PRIOR TO THE START OF CONSTRUCTION.
- 9. THE CONTRACTOR SHALL COORDINATE WITH ALL OWNERS TO DETERMINE THE LOCATION OF EXISTING LANDSCAPING, IRRIGATION LINES & PRIVATE UTILITY LINES. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO EXISTING LANDSCAPING, IRRIGATION LINES, AND PRIVATE UTILITY LINES.
- 10. THE CONTRACTOR SHALL REMOVE ALL TRASH AND DEBRIS FROM THE SITE UPON COMPLETION OF THE
- 11. THE CONTRACTOR SHALL MAINTAIN THE SITE IN A MANNER SO THAT WORKMEN AND PUBLIC SHALL BE PROTECTED FROM INJURY, AND ADJOINING PROPERTY PROTECTED FROM DAMAGE.
- 12. THE CONTRACTOR SHALL KEEP THE AREA OUTSIDE THE "CONSTRUCTION LIMITS" BROOM CLEAN AT ALL TIMES.
- 13. THE CONTRACTOR SHALL CALL MISS DIG A MINIMUM OF 72 HOURS PRIOR TO THE START OF CONSTRUCTION.
- 14. ALL PAVEMENT REPLACEMENT AND OTHER WORKS COVERED BY THESE PLANS SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE TOWNSHIP, INCLUDING THE LATEST MICHIGAN DEPARTMENT OF TRANSPORTATION (MDOT) SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.
- 15. THE CONTRACTOR IS RESPONSIBLE FOR ALL DAMAGE TO EXISTING UTILITIES.

ATTRACTIVE CONDITION AS ORIGINALLY DESIGNED AND CONSTRUCTED.

ENGINEER PRIOR TO WORK COMMENCING.

- 16. NO ADDITIONAL COMPENSATION WILL BE PAID TO THE CONTRACTOR FOR ANY DELAY OR INCONVENIENCE DUE TO THE MATERIAL SHORTAGES OR RESPONSIBLE DELAYS DUE TO THE OPERATIONS OF SUCH OTHER PARTIES DOING WORK INDICATED OR SHOWN ON THE PLANS OR IN THE SPECIFICATION OR FOR ANY REASONABLE DELAYS IN CONSTRUCTION DUE TO THE ENCOUNTERING OR EXISTING UTILITIES THAT MAY OR MAY NOT BE SHOWN ON THE PLANS.
- 17. DURING THE CONSTRUCTION OPERATIONS, THE CONTRACTOR SHALL NOT PERFORM WORK BY PRIVATE AGREEMENT WITH PROPERTY OWNERS ADJACENT TO THE PROJECT.
- 18. IF WORK EXTENDS BEYOND NOVEMBER 15, NO COMPENSATION WILL BE DUE TO THE CONTRACTOR FOR ANY WINTER PROTECTION MEASURES THAT MAY BE REQUIRED BY THE ENGINEER.
- 19. NO TREES ARE TO BE REMOVED UNTIL MARKED IN THE FIELD BY THE ENGINEER.
- 20. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO THE PROPERTY BEYOND THE CONSTRUCTION LIMITS INCLUDING BUT NOT LIMITED TO EXISTING FENCE, LAWN, TREES AND SHRUBBERY.
- 21. TRAFFIC SHALL BE MAINTAINED DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL SIGNS AND TRAFFIC CONTROL DEVICES. FLAG PERSONS SHALL BE PROVIDED BY THE CONTRACTOR IF DETERMINED NECESSARY BY THE ENGINEER. ALL SIGNS SHALL CONFORM TO THE MICHIGAN MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES AT NO COST TO THE TOWNSHIP. NO WORK SHALL BE DONE UNLESS THE APPROPRIATE TRAFFIC CONTROL DEVICES ARE IN PLACE.
- 22. ALL DEMOLISHED MATERIALS AND SOIL SPOILS SHALL BE REMOVED FROM THE SITE AT NO ADDITIONAL COST, AND DISPOSED OF IN ACCORDANCE WITH LOCAL, STATE AND FEDERAL REGULATIONS.
- 23. ANY EXISTING APPURTENANCES SUCH AS MANHOLES, GATE VALVES, ETC. SHALL BE ADJUSTED TO THE PROPOSED GRADE AND SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
- 24. ALL PERMANENT SIGNS AND PAVEMENT MARKINGS SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST REVISION OF THE MICHIGAN MUTCD MANUAL AND SHALL BE INCIDENTAL TO THE CONTRACT.
- 25. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL ITEMS REQUIRED FOR CONSTRUCTION OF THE PROJECT ARE INCLUDED IN THE CONTRACT. ANY ITEMS NOT SPECIFICALLY DESIGNATED IN THE PLANS SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
- 26. THE CONTRACTOR IS RESPONSIBLE FOR HAVING A SET OF APPROVED CONSTRUCTION PLANS. WITH THE LATEST REVISION DATE, ON SITE PRIOR TO THE START OF CONSTRUCTION. IN THE EVENT OF ANY QUESTIONS PERTAINING TO THE INTENT OF THE CONSTRUCTION PLANS OR SPECIFICATIONS, THE CONTRACTOR SHALL
- CONTACT THE DESIGN ENGINEER FOR A FINAL DETERMINATION FROM THE DESIGN ENGINEER. 27. THE CONTRACTOR, NOT THE OWNER OR THE ENGINEER, ARE RESPONSIBLE FOR THE MEANS, METHODS, AND SEQUENCE OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR SAFE EXECUTION OF THE PROJECT SCOPE IN ACCORDANCE WITH THE APPROVED CONSTRUCTION PLANS.
- 28. THE CONTRACTOR IS RESPONSIBLE FOR PRESERVING CONSTRUCTION STAKING AS NECESSARY. CONTRACTOR TO NOTIFY CONSTRUCTION SURVEYOR OF REPLACEMENT STAKES NEEDED WHICH SHALL BE AT THE CONTRACTORS
- 29. THE OWNER AND/OR CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING FRANCHISE UTILITY SERVICES (CABLE, ELECTRIC, GAS, ETC.) OWNER AND/OR CONTRACTOR SHALL WORK WITH UTILITY COMPANIES ON
- FURNISHING SITE UTILITY LAYOUTS AND PROVIDING CONDUIT CROSSINGS AS REQUIRED. 30. DAMAGE TO ANY EXISTING UTILITIES OR INFRASTRUCTURE (INCLUDING PAVEMENT, CURB, SIDEWALK, ETC.) SHALL PROMPTLY BE REPLACED IN KIND AND SHALL BE AT THE CONTRACTORS EXPENSE.
- 31. COORDINATION OF TESTING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND PER ALL CITY/TOWNSHIP/COUNTY REQUIREMENTS. COPIES OF ALL TEST REPORTS SHALL BE FURNISHED TO THE DESIGN
- 32. PRIOR TO THE START OF CONSTRUCTION, PROTECTION FENCING SHALL BE ERECTED AROUND THE TREE DRIPLINE OF ANY TREES INDICATED TO BE SAVED WITHIN THE LIMITS OF DISTURBANCE.
- 33. THE CONTRACTOR SHALL MAINTAIN DRAINAGE OF THE PROJECT AREA AND ADJACENT AREAS. WHERE EXISTING DRAINAGE FACILITIES ARE IMPACTED/DISTURBED DUE TO CONSTRUCTION, THE CONTRACTOR SHALL PROVIDE ANY NECESSARY TEMPORARY DRAINAGE PROVISIONS.
- 34. SOIL BORING LOGS ARE REPRESENTATIVE OF SPECIFIC POINTS ON THE PROJECT SITE, AND IF PROVIDED TO THE CONTRACTOR ARE FOR INFORMATIONAL PURPOSES ONLY.
- 35. WHERE CITY/TOWNSHIP STANDARD CONSTRUCTION DETAILS/SPECIFICATIONS ARE PROVIDED AND ARE IN CONFLICT WITH NOTES AND SPECIFICATIONS HEREIN, THE CITY/TOWNSHIP STANDARD SHALL GOVERN.

INDEMNIFICATION STATEMENT

CONTRACTOR TO FOLLOW MANUFACTURER

SPECS/RECOMMENDATIONS THAT SUPERCEDE PLANS

THE CONTRACTOR SHALL HOLD HARMLESS THE DESIGN PROFESSIONAL, MUNICIPALITY, COUNTY, STATE. AND ALL OF ITS SUB CONSULTANTS, PUBLIC AND PRIVATE UTILITY COMPANIES, AND LANDOWNERS FOR DAMAGES TO INDIVIDUALS AND PROPERTY, REAL OR OTHERWISE, DUE TO THE OPERATIONS OF THE CONTRACTOR AND/OR THEIR SUBCONTRACTORS.

GENERAL GRADING & SESC NOTES

- 1. THE CONTRACTOR SHALL HAVE IN PLACE ALL REQUIRED EROSION CONTROL METHODS AS INDICATED ON THE CONSTRUCTION PLANS AND AS REQUIRED BY GENERAL PRACTICE. SPECIFIC MEANS, METHODS AND SEQUENCES OF CONSTRUCTION MAY DICTATE ADDITIONAL SOIL EROSION CONTROL MEASURES BE NEEDED. THE CONTRACTOR SHALL COORDINATE WITH THE DESIGN ENGINEER ON THESE ANTICIPATED METHODS. ADDITIONAL SOIL EROSION CONTROL METHODS SHALL BE INCIDENTAL TO THE SCOPE OF WORK.
- 2. ACTUAL FIELD CONDITIONS MAY DICTATE ADDITIONAL OR ALTERNATE SOIL EROSION CONTROL MEASURES BE UTILIZED. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DEFICIENCIES OR FIELD CONDITIONS THAT WARRANT ADDITIONAL AND/OR ALTERNATIVE SESC MEASURES BE UTILIZED.
- 3. AT THE CLOSE OF EACH DAY, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING ALL CONSTRUCTION OPERATIONS, MATERIALS, DEBRIS, ETC ARE CONTAINED ON-SITE.
- 4. AT THE CLOSE OF EACH WORKING DAY, ALL DRAINAGE STRUCTURES SHALL BE FREE OF DIRT AND DEBRIS AT THE FLOW LINE.
- 5. ALL SOIL EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE PER MDEGLE REGULATIONS AND
- BEST PRACTICES, ALL SOIL EROSION CONTROL MEASURES SHALL BE MAINTAINED BY THE CONTRACTOR. THE SOIL EROSION CONTROL MEASURES SHALL BE KEPT IN PLACE UNTIL SUCH A TIME THAT THE SITE IS
- DETERMINED TO BE ESTABLISHED WITH ACCEPTABLE AMOUNT OF VEGETATIVE GROUND COVER.
- 7. ALL AREAS DISTURBED BY THE CONTRACTOR BEYOND THE NORMAL CONSTRUCTION LIMITS OF THE PROJECT SHALL BE SODDED OR SEEDED AS SPECIFIED OR DIRECTED BY THE ENGINEER.
- 8. AFTER REMOVAL OF TOPSOIL, THE SUBGRADE SHALL BE COMPACTED TO 95% OF ITS UNIT WEIGHT.
- 9. ALL GRADING IN THE PLANS SHALL BE DONE AS PART OF THIS CONTRACT. ALL DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SUBGRADE PRIOR TO COMPACTING.
- 10. ALL ROOTS, STUMPS AND OTHER OBJECTIONABLE MATERIALS SHALL BE REMOVED AND THE HOLE BACKFILLED WITH SUITABLE MATERIAL. WHERE GRADE CORRECTION IS REQUIRED, THE SUBGRADE SHALL BE CUT TO CONFORM TO THE CROSS-SECTION AS SHOWN IN THE PLANS.
- 11. ALL EXCAVATION UNDER OR WITHIN 3 FEET OF PUBLIC PAVEMENT, EXISTING OR PROPOSED SHALL BE BACKFILLED AND COMPACTED WITH SAND (MDOT CLASS II).

GENERAL LANDSCAPE NOTES

- 1. ALL PLANT MATERIAL SHALL CONFORM TO THE REQUIREMENTS AND SPECIFICATIONS OF THE GOVERNING MUNICIPALITY. ALL STOCK SHALL BE NURSERY GROWN, CONFORMING TO ANSI Z60.1 "AMERICAN STANDARD FOR NURSERY STOCK", AND IN ACCORDANCE WITH GOOD HORTICULTURAL PRACTICE. STOCK SHALL EXHIBIT NORMAL GROWTH HABIT AND BE FREE OF DISEASE, INSECTS, EGGS, LARVAE, & DEFECTS SUCH AS KNOTS, SUN-SCALD, INJURIES, ABRASIONS, OR DISFIGUREMENT. ALL PLANT MATERIAL SHALL BE SUBJECT TO THE APPROVAL OF THE LANDSCAPE ARCHITECT.
- ALL PLANT MATERIALS SHALL BE BALLED AND BURLAPPED OR CONTAINER STOCK. NO BARE ROOT STOCK IS PERMITTED. ALL PLANT BALLS SHALL BE FIRM, INTACT, AND SECURELY WRAPPED AND BOUND.
- 3. ALL PLANT BED MATERIALS SHALL BE EXCAVATED OF ALL BUILDING MATERIALS, OTHER EXTRANEOUS OBJECTS, AND POOR SOILS TO A MINIMUM DEPTH OF 12-INCHES AND BACKFILLED TO GRADE WITH SPECIFIED PLANTING MIX (SEE BELOW).
- 4. PLANTING MIXTURE SHALL CONSIST OF 5 PARTS TOPSOIL FROM ON-SITE (AS APPROVED), 4 PARTS COARSE SAND, 1 PART SPHAGNUM PEAT MOSS (OR APPROVED COMPOST), AND 5 LBS OF SUPERPHOSPHATE FERTILIZER PER CU. YD. OF MIX. INGREDIENTS SHALL BE THOROUGHLY BLENDED FOR UNIFORM CONSISTENCY.
- 5. ALL PLANT BEDS AND INDIVIDUAL PLANTS, NOT OTHERWISE NOTED SHALL BE MULCHED WITH A 4-INCH LAYER OF SHREDDED BARK MULCH. EDGE OF MULCH BEDS AS SHOWN. DECIDUOUS TREES IN LAWN AREAS SHALL RECEIVE A 5-FT DIAMETER CIRCLE OF MULCH AND CONIFER TREES 8-FT (PLANTED CROWN OF TREE) UNLESS OTHERWISE NOTED.
- LANDSCAPE STONE SHALL BE INSTALLED WHERE NOTED OR INDICATED (HATCHED). STONE SHALL BE 3/4"-1-1/4" WASHED RIVER GRAVEL OR AS SELECTED AND SHALL BE INSTALLED TO A MINIMUM DEPTH OF 3-INCHES.
- 7. ALL LANDSCAPE BEDS, UNLESS OTHERWISE NOTED SHALL BE INSTALLED OVER WEED BARRIER FABRIC WATER PERMEABLE FILTRATION FABRIC OF NON-WOVEN POLYPROPYLENE OR POLYESTER FABRIC. FABRIC SHALL BE OF SUITABLE THICKNESS FOR APPLICATION.
- 8. ALL PLANTS AND PLANT BEDS SHALL BE THOROUGHLY WATERED UPON COMPLETION OF PLANTING AND STAKING OPERATIONS.
- THE CONTRACTOR SHALL GUARANTEE ALL PLANT MATERIALS FOR A PERIOD OF 1 YEAR FROM THE DATE THE WORK IS ACCEPTED, IN WRITING, BY THE LANDSCAPE ARCHITECT. THE CONTRACTOR SHALL REPLACE, WITHOUT COST TO THE OWNER, WITHIN A SPECIFIED PERIOD OF TIME, ALL DEAD PLANTS, AND ALL PLANTS NOT IN A VIGOROUS, THRIVING CONDITION, AS DETERMINED BY THE LANDSCAPE ARCHITECT, DURING AND AT THE END OF THE GUARANTEE PERIOD. REPLACEMENT STOCK SHALL CONFORM TO THE ORIGINAL SPECIFICATIONS.
- 10. EDGING SHALL BE PROVIDED FOR ALL LANDSCAPE BEDS NOT ADJACENT TO CONCRETE PAVEMENT. EDGING SHALL BE BLACK ALUMINUM EDGING. 3/16-INCH X 4-INCH. INSTALL PER MANUFACTURER'S INSTRUCTIONS. ALL EDGING SHALL BE INSTALLED IN STRAIGHT LINES OR SMOOTH CURVES WITHOUT IRREGULARITIES.
- 11. SOD SHALL BE DENSE, WELL ROOTED TURF, FREE OF WEEDS. IT SHALL BE COMPRISED OF A BLEND OF AT LEAST TWO KENTUCKY BLUE GRASSES AND ONE FESCUE. IT SHALL HAVE A UNIFORM THICKNESS OF 3/4-INCH AT TIME OF PLANTING, AND CUT IN UNIFORM STRIPS NOT LESS THAN 10-INCHES BY 18-INCHES. SOD SHALL BE KEPT MOIST AND LAID WITHIN 36-HOURS AFTER CUTTING.
 - IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH A DENSE LAWN OF PERMANENT GRASSES, FREE OF LUMPS AND DEPRESSIONS. ALL SODDED AREAS THAT BROWN-OUT OR HAVE NOT FIRMLY KNITTED TO THE SOIL BASE WITHIN A PERIOD OF 1 MONTH SHALL BE REPLACED BY THE CONTRACTOR, AT NO COST TO THE OWNFR.
- 12. ALL AREAS OF THE SITE THAT BECOME DISTURBED DURING CONSTRUCTION AND ARE NOT TO BE PAVED, STONED, LANDSCAPED, OR SODDED SHALL BE SEEDED AND MULCHED.

SEED MIXTURE SHALL BE AS FOLLOWS: KENTUCKY BLUEGRASS (CHOOSE 3 VARIETIES -30% ADELPHI, RUGBY, GLADE, OR PARADE) RUBY RED OR DAWSON RED FINE FESCUE 30% ATLANTA RED FESCUE 20% PENNFINE PERENNIAL RYE 20%

THE ABOVE SEED MIXTURE SHALL BE SOWN AT A RATE OF 250 LBS PER ACRE. PRIOR TO SEEDING, THE TOPSOIL SHALL BE FERTILIZED WITH A COMMERCIAL FERTILIZER WITH A 10-0-10 ANALYSIS:

10% NITROGEN - MIN 25% FROM A UREA FORMALDEHYDE SOURCE

0 % PHOSPHATE 10% POTASH - SOURCE POTASSIUM SULFATE OR POTASSIUM NITRATE

THE FIRST FERTILIZER APPLICATION SHALL BE AT A RATE OF 10 LBS PER 1000 SQ FT OF BULK FERTILIZER.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH A DENSE LAWN OF PERMANENT GRASSES, FREE OF LUMPS AND DEPRESSIONS. ANY PART OF THE AREA THAT FAILS TO SHOW A UNIFORM GERMINATION SHALL BE RE-SEEDED AND SUCH RE-SEEDING SHALL CONTINUE UNTIL A DENSE LAWN IS ESTABLISHED. DAMAGE TO SEEDED AREAS RESULTING FROM EROSION SHALL BE REPAIRED BY THE CONTRACTOR.

- 13. ALL AREAS OF THE SITE SCHEDULED FOR SEEDING OR SODDING SHALL FIRST RECEIVE A 6-INCH LAYER OF CLEAN, FRIABLE TOPSOIL. THE SOIL SHALL BE DISCED AND SHALL BE GRADED IN CONFORMANCE WITH THE GRADING PLAN.
- 14. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION OF ALL UTILITIES AND TO INFORM THE LANDSCAPE ARCHITECT OF ANY CONFLICTS PRIOR TO COMMENCING LANDSCAPING.

GENERAL UTILITY NOTES

- BEDDING SHALL EXTEND A MINIMUM OF 4" BELOW THE PIPE, UNLESS OTHERWISE NOTED ON THE PLANS. BEDDING SHALL BE OF UNIFORM GRADATION MDOT 6AA STONE OR MDOT CLASS II GRANULAR MATERIAL FOR SANITARY AND STORM PIPE AND MDOT CLASS II GRANULAR MATERIAL ONLY FOR WATERMAIN.
- 2. WHERE UNSTABLE GROUND CONDITIONS ARE ENCOUNTERED, STONE BEDDING SHALL BE USED AS DIRECTED BY THE ENGINEER.
- 3. BACKFILL SHALL BE OF A SUITABLE MATERIAL AND SHALL BE FREE OF ANY ORGANIC MATERIALS AND ROCKS.
- 4. BACKFILL ABOVE THE PIPE SHALL BE OF GRANULAR MATERIAL MDOT CLASS II TO A POINT 12" ABOVE THE TOP OF THE PIPE. WHERE THE TRENCH IS NOT WITHIN THE INFLUENCE OF THE ROAD, SUITABLE SITE MATERIAL MAY BE COMPACTED AND UTILIZED FROM A POINT 12" ABOVE THE PIPE TO GRADE. WHERE THE TRENCH IS WITHIN A 1:1 INFLUENCE OF THE ROAD, GRANULAR MATERIAL, MDOT CLASS II OR III, IS TO BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 12" IN THICKNESS. COMPACTION SHALL BE 95% AS DETERMINED BY AASHTO T99.
- 5. 18" MINIMUM VERTICAL SEPARATION AND 10' HORIZONTAL SEPARATION IS TO BE MAINTAINED BETWEEN WATERMAIN AND SANITARY/STORM SEWER TO THE MAXIMUM EXTENT POSSIBLE.

GENERAL STORM NOTES

- 1. ALL STORM PIPE LENGTHS ARE SHOWN FROM C/L TO C/L OF STRUCTURE OR FROM C/L OF STRUCTURE TO DISCHARGE END OF FLARED END SECTION.
- 2. STORM PIPE MATERIALS SHALL BE AS FOLLOWS: 2.1. RCP(REINFORCED CONCRETE PIPE): SHALL MEET THE REQUIREMENTS OF ASTM C76 WITH MODIFIED GROOVED TONGUE AND RUBBER GASKETS MEETING THE REQUIREMENTS OF ASTM C443. RCP TO BE EITHER CLASS IV OR V AS CALLED OUT ON THE PLANS.
- 3. STORM PIPE JOINTS SHALL MEET THE REQUIREMENTS OF ASTM D3212. HDPE AND PP PIPE GASKETS SHALL MEET THE REQUIREMENTS OF ASTM F477.
- 4. ALL STORM PIPE TO HAVE WATERTIGHT PREMIUM JOINTS, UNLESS OTHERWISE NOTED ON THE PLANS.
- 5. STORM DRAINAGE STRUCTURES SHALL BE FURNISHED WITH STEPS WHICH SHALL BE STEEL ENCASED WITH POLYPROPYLENE PLASTIC OR EQUIVALENT. STEPS SHALL BE SET AT 16" CENTER TO CENTER.
- 6. FLARED END SECTIONS DISCHARGING STORM WATER SHALL RECEIVE A MINIMUM OF 10 SQ YDS OF PLAIN COBBLESTONE RIP RAP WITH A MINIMUM STONE SIZE OF 6" AND SHALL BE PLACED ON A GEOTEXTILE FABRIC
- 7. ALL CATCH BASINS WITHIN THE ROADWAY SHALL INCLUDE INSTALLATION OF 6" DIAMETER PERFORATED PIPE SUBDRAIN.
- 8. STORM DRAINAGE STRUCTURE COVERS SHALL BE OF THE FOLLOWING (OR APPROVED EQUAL): COVER USE FRAME GRATE/BACK
 - 1040 TYPE 'B' MANHOLE PARKING LOTS 1040/5100 TYPE 'M1' GRATE OR 5105 TYPE 'M1' GRATE LAWN 1040 TYPE '02' GRATE

GENERAL SANITARY NOTES

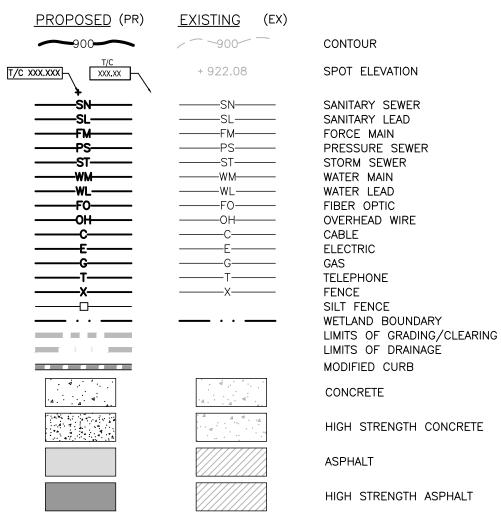
- 1. ALL SANITARY PIPE LENGTHS ARE SHOWN FROM C/L OF STRUCTURE TO C/L OF STRUCTURE.
- 2. SANITARY PIPE MATERIALS SHALL BE AS FOLLOWS: 2.1. HDPE DR-11 (SANITARY FORCEMAIN)
- 3. ALL PVC SDR SANITARY SEWER PIPE SHALL MEET THE REQUIREMENTS OF ASTM D3034 AND D2241. PVC SCHD 40 PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785. GASKET JOINTS FOR SANITARY PIPE SHALL MEET THE REQUIREMENTS OF ASTM D3139 AND D3212.
- 4. SANITARY STRUCTURES SHALL BE FURNISHED WITH STEPS WHICH SHALL BE STEEL ENCASED WITH POLYPROPYLENE PLASTIC OR EQUIVALENT. STEPS SHALL BE SET AT 16" CENTER TO CENTER.
- ALL NEW MANHOLES SHALL BE MINIMUM 4' DIAMETER, PRECAST MANHOLE SECTIONS AND AN ECCENTRIC CONE. PRECAST MANHOLE JOINTS SHALL BE INSTALLED WITH BUTYL ROPE MEETING THE REQUIREMENTS OF ASTM C990.
- 6. MANHOLES SHALL BE CONSTRUCTED WITH FLOW CHANNEL WALLS THAT ARE FORMER, AT A MINIMUM, TO THE SPRINGLINE OF THE PIPE.
- 7. ALL NEW MANHOLES SHALL HAVE AN APPROVED FLEXIBLE, WATERTIGHT SEALS WHERE PIPES PASS THROUGH
- 8. ALL MANHOLES SHALL BE PROVIDED WITH WATERTIGHT COVERS. COVERS TO BE EJCO 1040 TYPE 'A' SOLID
- 9. A MAXIMUM OF 12" OF GRADE ADJUSTMENT RINGS SHALL BE USED TO ADJUST THE FRAME ELEVATION. BUTYL ROPE SHALL BE USED BETWEEN EACH ADJUSTMENT RING.
- 10. SANITARY SEWER LATERALS SHALL HAVE A MINIMUM SLOPE OF 1.0%.
- 11. CLEANOUTS SHALL BE INSTALLED EVERY 100', AT ALL BENDS AND STUBS.

GENERAL WATERMAIN NOTES

- 1. WATERMAIN PIPE MATERIALS SHALL BE AS FOLLOWS:
- 1.1. D.I.P. CL.52 (WATERMAIN) 1.2. HDPE DR-9 (WATER LATERAL - CURB STOP TO STUB)
- 2. WATERMAIN FITTINGS SHALL BE OF DUCTILE IRON WITH CEMENT MORTAR LINING AND MECHANICAL JOINTS CONFORMING TO AWWA C110.
- WATERMAINS SHALL BE DISINFECTED IN ACCORDANCE WITH AWWA C651. BAC-T SAMPLES SHALL BE TAKEN IN ACCORDANCE WITH R235.11110 OF THE ADMINISTRATIVE RULES PROMULGATED UNDER MICHIGAN SAFE DRINKING WATER ACT, 1976 PA 399, AS AMENDED.
- 4. ALLOWABLE LEAKAGE OR HYDROSTATIC PRESSURE TESTING SHALL BE IN ACCORDANCE WITH AWWA C600 AND
- MAXIMUM DEFLECTION AT PIPE JOINTS SHALL BE IN ACCORDANCE WITH PIPE MANUFACTURERS CURRENT RECOMMENDATIONS AND AWWA SPECIFICATIONS.
- 6. A FULL STICK OF PIPE SHALL BE LAID CENTERED AT A PIPE CROSSING IN ORDER TO MAINTAIN THE MAXIMUM SEPARATION OF WATERMAIN JOINT TO THE CROSSING PIPE. 7. WATERMAIN SHALL BE INSTALLED WITH A MINIMUM OF 5.5' OF COVER FROM FINISHED GRADE TO TOP OF PIPE
- 8. WATERMAIN VALVES SHALL BE IRON BODY RESILIENT WEDGE GATE VALVES, NON-RISING STEMS, COUNTERCLOCKWISE OPEN, AWWA C509.
- 9. THE BREAKAWAY FLANGE AND ALL BELOW GRADE FITTINGS SHALL HAVE STAINLESS STEEL NUTS AND BOLTS.

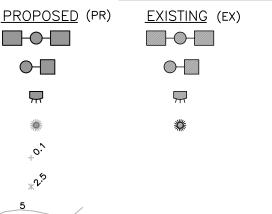
AND NO MORE THAN 8' OF COVER. UNLESS SPECIAL CONDITIONS WARRANT.

LINES & HATCHES LEGEND



SOIL EROSION CONTROL MEASURE (P=PERMANENT, T=TEMPORARY)

LIGHTING LEGEND



DOUBLE FIXTURE LIGHT POLE SINGLE FIXTURE LIGHT FIXTURE WALL MOUNTED LIGHT FIXTURE

GROUND LIGHT FIXTURE FOOT CANDLES ON SITE FOOT CANDLES OFF SITE

WETLAND

SANITARY SEWER LABEL

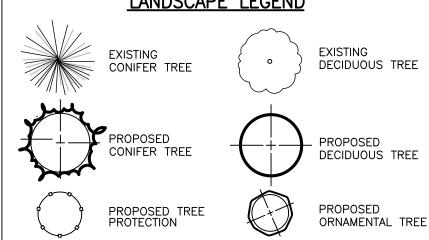
STORM SEWER LABEL

WATER MAIN LABEL

FOOT CANDLES CONTOURS CANOPY MOUNTED LIGHT FIXTURE

LANDSCAPE LEGEND

(54T)



L.C.R.



DECIDUOUS SHRUBS PROPOSED GRASSES & PERENNIALS PROPOSED

LANDSCAPE BOULDER

<u>ABBREVIATIONS</u>

FINISHED FLOOR ELEVATION BASEMENT FLOOR FLEVATION GARAGE FLOOR ELEVATION FINISHED GRADE TOP OF ASPHALT TOP OF CONCRETE/CURB TOP OF WALK TOP OF PIPE BOTTOM OF PIPE FLOW LINE RIM ELEVATION (AT FLOW LINE) INVERT ELEVATION MANHOLE CATCH BASIN REAR YARD YARD DRAIN FLARED END SECTION CORRUGATED METAL PIPE CORRUGATED PLASTIC PIPE REINFORCED CONCRETE PIPE HIGH DENSITY POLYETHYLENE POLYVINYL CHLORIDE DUCTILE IRON PIPE GATE VALVE GATE VALVE IN WELL GATE VALVE IN BOX FIRE DEPARTMENT CONNECTION UTILITY POLE NOT FIELD VERIFIED TO BE REMOVED

LIVINGSTON COUNTY RECORDS

MEASURED AND RECORD

POINT OF BEGINNING

SYMBOL LEGEND

STORM DRAINAGE FLOW ↓ GUY WIRE -∽ POWER POLE TRANSFORMER PAD E ELECTRICAL RISER E U.G. ELECTRIC MARKER ELECTRICAL METER AIR CONDITIONING UNIT

TELEPHONE RISER U.G. TELEPHONE MARKER G GAS RISER U.G. GAS MARKER GAS METER

CABLE TV RISER MB MAILBOX WELL

W WATER MANHOLE Ø GATE VALVE (EXISTING)

-CX HYDRANT (EXISTING) → HYDRANT (PROPOSED)

STORM MANHOLE (PROPOSED) (END SECTION (EXISTING) ♠ END SECTION (PROPOSED) SANITARY MANHOLE (EXISTING)

SANITARY MANHOLE (PROPOSED)

SIGN (PROPOSED) SOIL BORING O STEEL ROD SET ☐ WOOD LATH SET ☐ HUB SET

GP GAS PUMP ANTENNA (XX) SATELLITE DISH MP NEWSPAPER BOX

BENCHMARK

LIGHT POLE

AS SI GUARA COMP COMP LOCAT UTILIT CONTE FROM

Ш

ΙШ B

U.G. CABLE TV MARKER

☐ CATCH BASIN (EXISTING)

CATCH BASIN (PROPOSED) O STORM MANHOLE (EXISTING)

PC PUMP CHAMBER

-ф- TRAFFIC SIGN

STEEL ROD OR PIPE FOUND

MONUMENT FOUND SECTION CORNER

PM PARKING METER PB PHONE BOOTH HANDICAP SYMBOL

05/31/24

JOB NO: **24-048**

ESIGNED BY:

CHECKED BY:

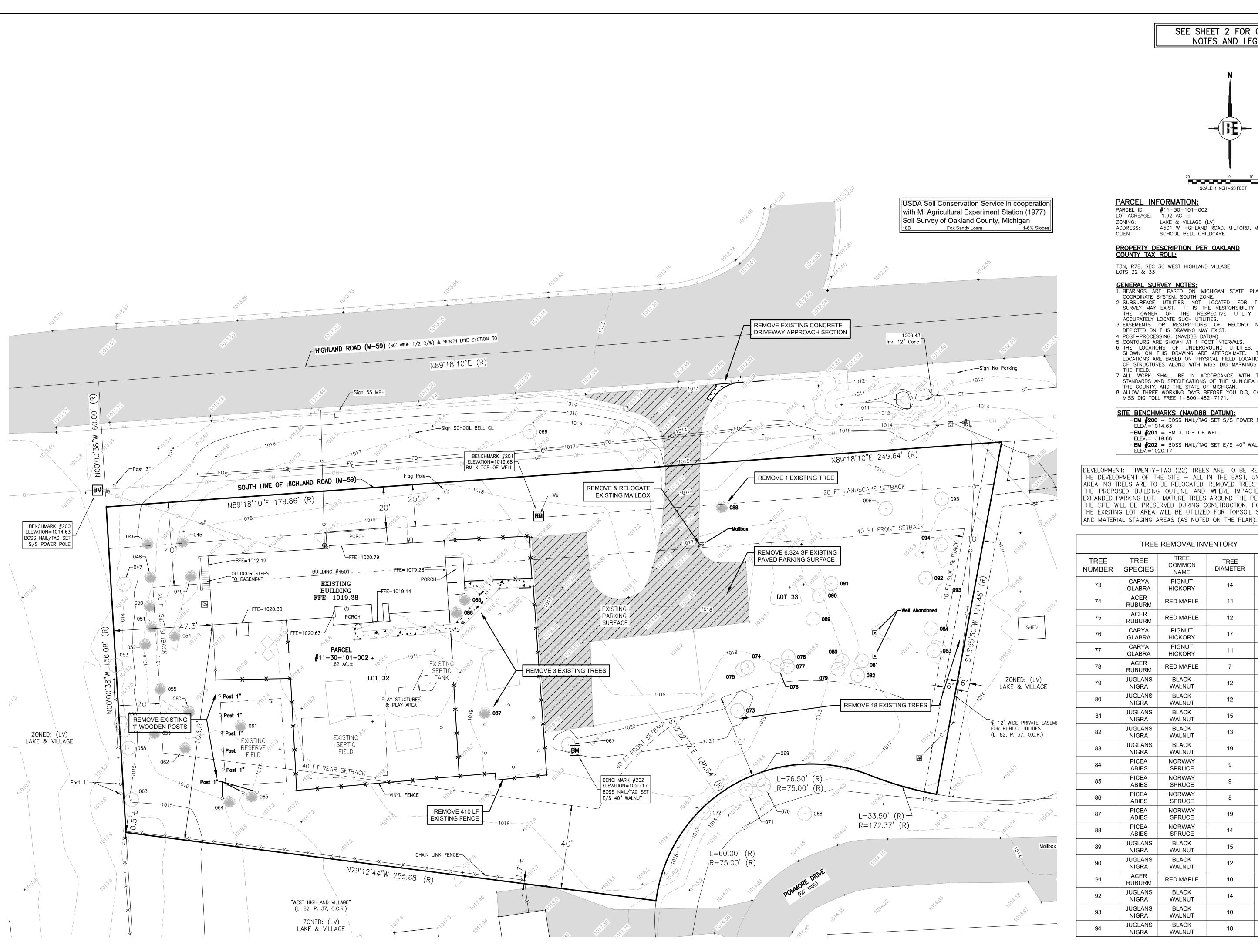
JP

NO SCALE

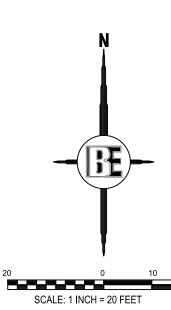
DRAWN BY:

SCALE:

 $\label{eq:control_co$



SEE SHEET 2 FOR GENERAL NOTES AND LEGEND



PARCEL INFORMATION: PARCEL ID: #11-30-101-002 LOT ACREAGE: 1.62 AC. ±

ZONING: LAKE & VILLAGE (LV) 4501 W HIGHLAND ROAD, MILFORD, MI 48380 ADDRESS: SCHOOL BELL CHILDCARE

PROPERTY DESCRIPTION PER OAKLAND COUNTY TAX ROLL:

T3N, R7E, SEC 30 WEST HIGHLAND VILLAGE LOTS 32 & 33

GENERAL SURVEY NOTES: 1. BEARINGS ARE BASED ON MICHIGAN STATE PLANE COORDINATE SYSTEM, SOUTH ZONE.

- 2. SUBSURFACE UTILITIES NOT LOCATED FOR THIS SURVEY MAY EXIST. IT IS THE RESPONSIBILITY OF THE OWNER OF THE RESPECTIVE UTILITY TO ACCURATELY LOCATE SUCH UTILITIES.

 3. EASEMENTS OR RESTRICTIONS OF RECORD NOT DEPICTED ON THIS DRAWING MAY EXIST. 4. POST-PROCESSING. (NAVD88 DATUM)
- 5. CONTOURS ARE SHOWN AT 1 FOOT INTERVALS. 6. THE LOCATIONS OF UNDERGROUND UTILITIES, AS SHOWN ON THIS DRAWING ARE APPROXIMATE. THE LOCATIONS ARE BASED ON PHYSICAL FIELD LOCATIONS OF STRUCTURES ALONG WITH MISS DIG MARKINGS IN THE FIELD.

 7. ALL WORK SHALL BE IN ACCORDANCE WITH THE
- STANDARDS AND SPECIFICATIONS OF THE MUNICIPALITY, THE COUNTY, AND THE STATE OF MICHIGAN. 8. ALLOW THREE WORKING DAYS BEFORE YOU DIG, CALL MISS DIG TOLL FREE 1-800-482-7171.

SITE BENCHMARKS (NAVD88 DATUM):

-BM #200 = BOSS NAIL/TAG SET S/S POWER POLE ELEV.=1014.63 -BM #201 = BM X TOP OF WELL ELEV.=1019.68 -BM #202 = BOSS NAIL/TAG SET E/S 40" WALNUT

DEVELOPMENT: TWENTY-TWO (22) TREES ARE TO BE REMOVED FOR THE DEVELOPMENT OF THE SITE - ALL IN THE EAST, UNDEVELOPED AREA. NO TREES ARE TO BE RELOCATED. REMOVED TREES ARE WITHIN THE PROPOSED BUILDING OUTLINE AND WHERE IMPACTED BY THE EXPANDED PARKING LOT. MATURE TREES AROUND THE PERIMETER OF THE SITE WILL BE PRESERVED DURING CONSTRUCTION. PORTIONS OF THE EXISTING LOT AREA WILL BE UTILIZED FOR TOPSOIL STOCKPILING

TREE REMOVAL INVENTORY					
TREE NUMBER	TREE SPECIES	TREE COMMON NAME	TREE DIAMETER	TREE CONDITION	
73	CARYA GLABRA	PIGNUT HICKORY	14	FAIR	
74	ACER RUBURM	RED MAPLE	11	FAIR	
75	ACER RUBURM	RED MAPLE	12	FAIR	
76	CARYA GLABRA	PIGNUT HICKORY	17	FAIR	
77	CARYA GLABRA	PIGNUT HICKORY	11	FAIR	
78	ACER RUBURM	RED MAPLE	7	POOR	
79	JUGLANS NIGRA	BLACK WALNUT	12	POOR	
80	JUGLANS NIGRA	BLACK WALNUT	12	POOR	
81	JUGLANS NIGRA	BLACK WALNUT	15	POOR	
82	JUGLANS NIGRA	BLACK WALNUT	13	POOR	
83	JUGLANS NIGRA	BLACK WALNUT	19	FAIR	
84	PICEA ABIES	NORWAY SPRUCE	9	GOOD	
85	PICEA ABIES	NORWAY SPRUCE	9	GOOD	
86	PICEA ABIES	NORWAY SPRUCE	8	GOOD	
87	PICEA ABIES	NORWAY SPRUCE	19	GOOD	
88	PICEA ABIES	NORWAY SPRUCE	14	GOOD	
89	JUGLANS NIGRA	BLACK WALNUT	15	GOOD	
90	JUGLANS NIGRA	BLACK WALNUT	12	POOR	
91	ACER RUBURM	RED MAPLE	10	FAIR	
92	JUGLANS NIGRA	BLACK WALNUT	14	POOR	
93	JUGLANS NIGRA	BLACK WALNUT	10	FAIR	
94	JUGLANS NIGRA	BLACK WALNUT	18	POOR	

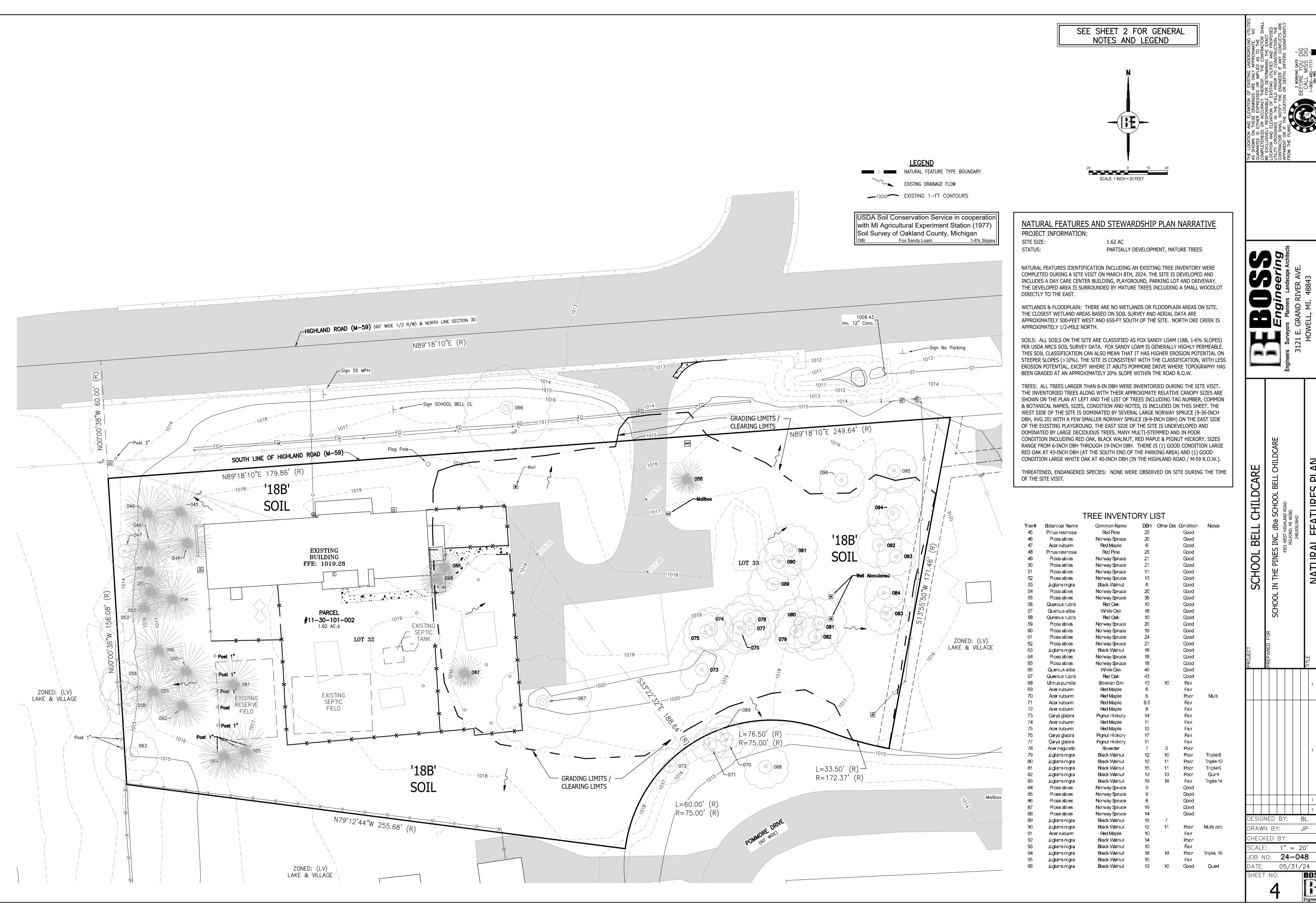
THE L AS SE COMPL COMPL LOCATI UTILITY CONTR APPAR

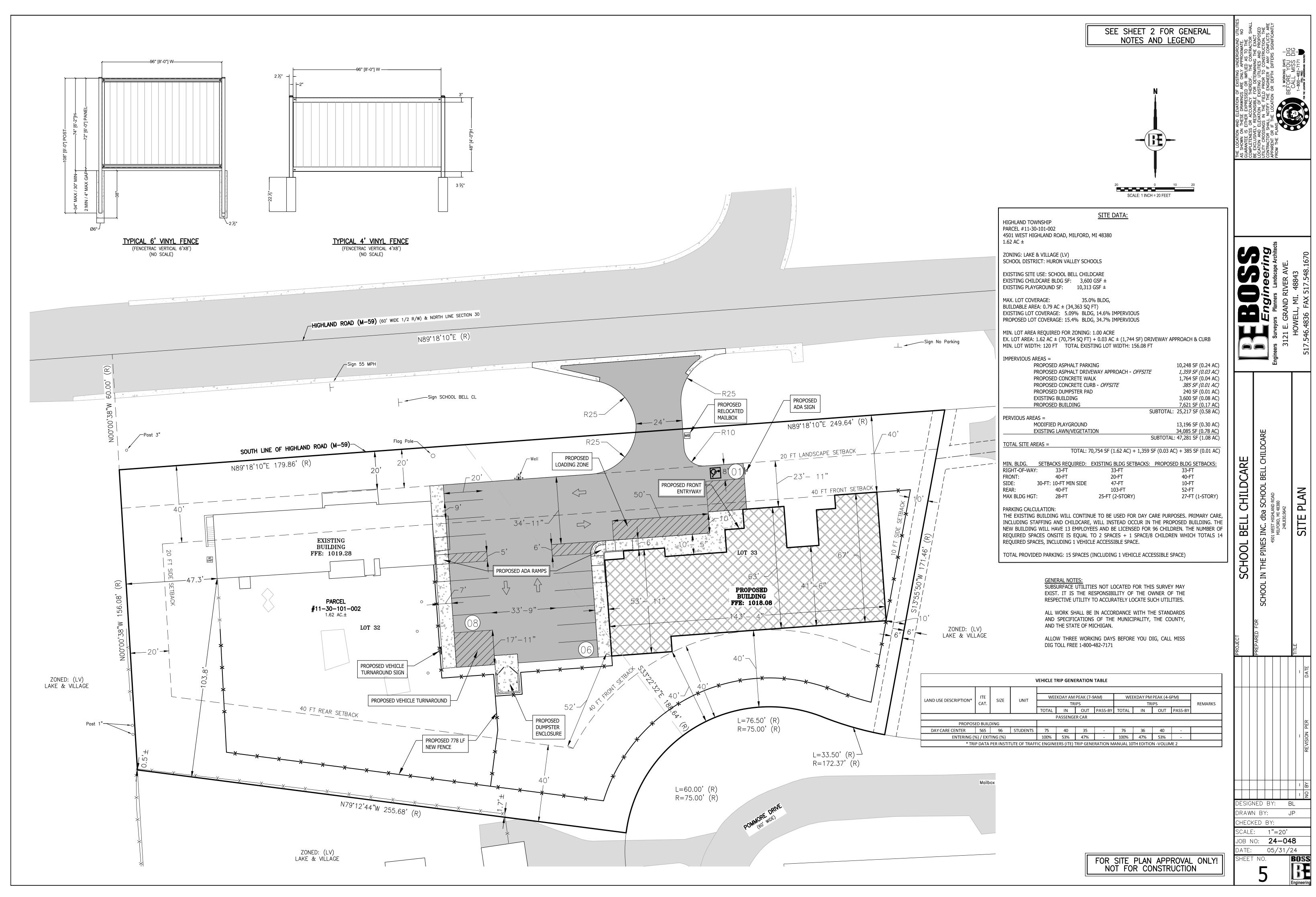
NOILITC

EXISTING

BELL

DRAWN BY: CHECKED BY: 1"=20' JOB NO: **24-048** 05/31/24





SITE BENCHMARKS (NAVD88 DATUM): -BM #200 = BOSS NAIL/TAG SET S/S POWER POLE ELEV.=1014.63 -BM #201 = BM X TOP OF WELL ELEV.=1019.68

TIME OF ADD'L RUNOFF PIPE PIPE VELOCITY HYDRAULIC ACTUAL

DIA.

(IN)

12

12

ELEV.=1020.17

LENGTH

(LF)

28

EQUIV.

A * C

0.11

0.07 4.336

INTENSITY

4.375

CONC.

15.00

15.36

RUNOFF

(CFS)

Q

RUNOFF

COEFF

0.678

0.793

BASIN - - - 15.00

100-YEAR PIPE FLOW CALCULATION FOR OCS

DRAIN ACRES

0.082

AREA

4

3

FROM

TO

BASIN

-BM #202 = BOSS NAIL/TAG SET E/S 40" WALNUT

FLOWING

FULL (FPS)

1.80

1.99

1.99

GRADIENT

SLOPE %

0.16%

0.19%

0.19%

SLOPE

USED

0.32%

0.32%

0.32%

OUTFLOW COMPARATIVE TABLE					
DRAINAGE AREA (CFS)	2-YR STORM	10-YR STORM	50-YR STORM	100-YR STORM	
EXISTING	0.73	1.28	2.09	2.5	
PROPOSED	0.034	0.042	0.05	0.147	
TO REMAIN	0.56	0.93	1.46	1.73	
IMPROVEMENT	0.136	0.308	0.58	0.623	

FLOW

CAPACITY

2.02

2.02

2.02

MANNING

COEFFICIENT

0.013

0.013

0.013

MANNING MANNING'S

VELOCITY

(FT/SEC)

2.57

2.57

2.57

TIME

(MIN)

UPPER

END

LOWER

END

0.05 1013.47 1013.44 1017.16

0.18 1010.56 1010.47 -

STORM WATER MANAGEMENT NARRATIVE:

HG ELEV HG ELEV RIM ELEV RIM ELEV INVERT INVERT

END

0.36 1013.64 1013.47 1017.20 1017.16 1012.84 1012.67

THE STORM WATER MANAGEMENT OF THE SITE WILL INCLUDE AN ONSITE STORM SEWER NETWORK LEADING TO AN UNDERGROUND DETENTION SYSTEM. THE UNDERGROUND DETENTION SYSTEM WILL BE CONSTRUCTED TO CAPTURE AND DETAIN THE 100-YEAR STORM EVENT WHICH WILL RESULT IN A LOWER RATE OF DISCHARGE TO THE WEST HIGHLAND ROAD (M-59) RIGHT-OF-WAY THAN THE EXISTING CONDITIONS CURRENTLY ALLOW.

END

UPPER LOWER

1012.67 1012.64

END

END

1016.80 1009.76 1009.67

STORMWATER RUNOFF SUMMARY:

EXISTING PEAK OUTFLOW RATES 100-YR 2-YR 10-YR 50-YR

POST-DEVELOPMENT PEAK RUNOFF STORM EVENT

2-YR 10-YR 50-YR 100-YR 0.034 CFS 0.042 CFS 0.050 CFS 0.147 CFS

EXISTING RUNOFF TO REMAIN FROM STORM EVENT

0.560 CFS 0.930 CFS 1.46 CFS 1.73 CFS

50-YR 100-YR

COMPARING ALL POST-DEVELOPMENT PEAK RUNOFF FLOWS TO THE PRE-DEVELOPMENT PEAK RUNOFF FLOWS, ALL STORM EVENTS SEE A REDUCTION IN STORMWATER FLOW TO M-59. SEE BELOW FOR REDUCTION OF FLOWS.

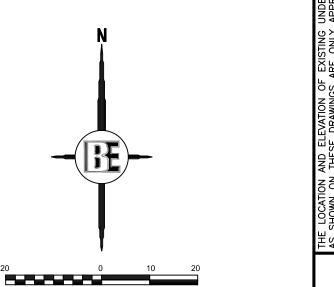
REDUCTION OF FLOW: 2-YR 10-YR

2-YR 10-YR

50-YR 100-YR



SCALE: 1 INCH = 20 FEET



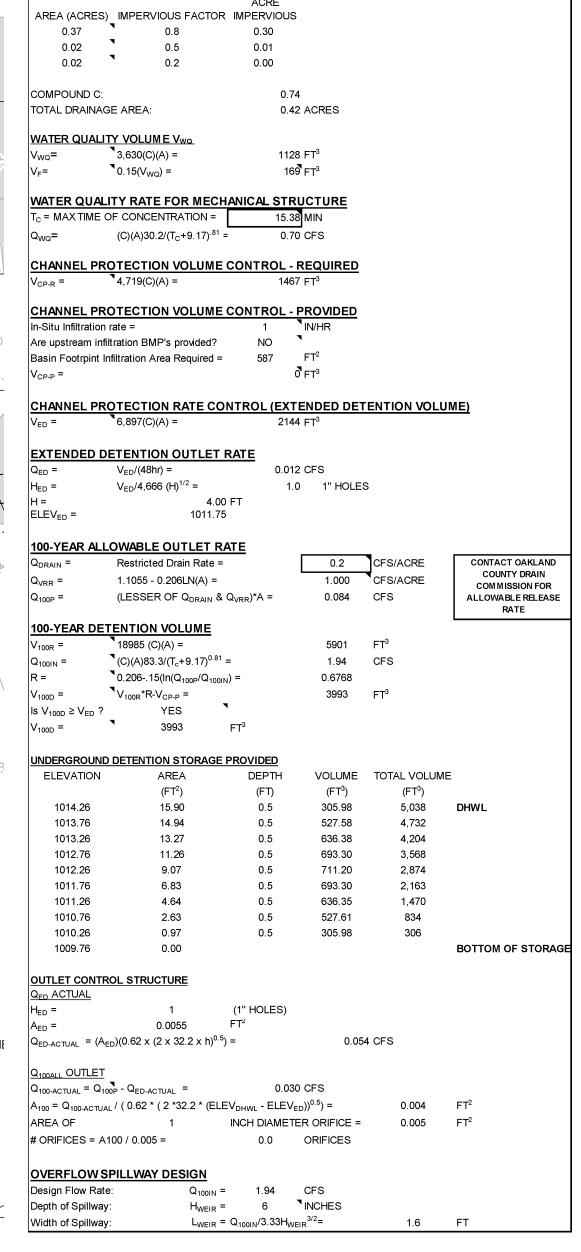


PLAN 8 CHILD BELL

ESIGNED BY: DRAWN BY: CHECKED BY:

SCALE: 1"=20' JOB NO: **24-048** 05/31/24

2 1 ALL 0.42 0.75 0.31 6.24 15.36 1.95 54 15	2.35 0.20% 0.24% 0.013 3.17	2.59 0.35 1010.67 1010.54 1016.80	- 1009.67 1009.54 0.136 CFS	0.308 CFS
		3.6	1072.	OAKLAND COUNTY DETENTION BASIN CALCULATIONS ACRE
	N13.15A	,0		AREA (ACRES) IMPERVIOUS FACTOR IMPERVIOUS 0.37
	4		1012.86 11.88	0.02 0.2 0.00 COMPOUND C: 0.74
10131A	N3.15		40	TOTAL DRAINAGE AREA: 0.42 ACRES
162		01	1009.43 Inv. 12" Conc.	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
			83 N.2.5h	WATER QUALITY RATE FOR MECHANICAL STRUCTURE T _C = MAX TIME OF CONCENTRATION = 15.38 MIN
	3501	x 1012.88 1012.	1012 + INV 1009.54 3 6 - 100	Sign No Parking $Q_{WQ} = (C)(A)30.2/(T_C + 9.17)^{.81} = 0.70 \text{ CFS}$
Sign 55 MPH	1013.26 1011	1013	1011 (FES) [1009.34] (+ ST	ST CHANNEL PROTECTION VOLUME CONTROL - REQUIRED V _{CP-R} = 4,719(C)(A) = 1467 FT ³
101519	1014.	1014		In-Situ Infiltration rate = 1 IN/HR Are upstream infiltration BMP's provided?
3	Sign SCHOOL BELL CL 066	+015.33 +016.1 +015.2 +016.1 +013.3 +016.1 +015.2 +016.1 +016.2 +016.1 +016.2 +	+ + OH = 1015 - 1014 H + OH = 1015	Basin Footrpint Infiltration Area Required = 587 FT² V _{CP-P} = 0 FT³
+1013. K 1015. K 1015. K 1015. K 1015. K 1017. TO 17.5	DENCIMARIA #201	OH 0H	1,016.0	CHANNEL PROTECTION RATE CONTROL (EXTENDED DET V _{ED} = \$\frac{1}{9},897(C)(A) = 2144 FT^3\$
Post 3" $FQ = FQ$ $FQ = FQ$ $OH = G$	BENCHMARK #201 ELEVATION=1019.68 BM X TOP OF WELL	+ 1015 MB T/A	70 ₁₆	$\frac{\text{EXTENDED DETENTION OUTLET RATE}}{Q_{\text{ED}} = V_{\text{ED}}/(48\text{hr}) = 0.012 \text{ CFS}}$
— он — — — он — — — он — — — он — — — он — — — он — — — он — — — —		T/A 1017.07 1016 1017.07	OCS RIM 1016.80	$H_{ED} = V_{ED}/4,666 \text{ (H)}^{1/2} = 1.0 \text{ 1" HOLES}$ $H = 4.00 \text{ FT}$ $ELEV_{ED} = 1011.75$
-OH-	T/A 1018.59	3,27% + 1017	T/A 096 095 FG	100-YEAR ALLOWABLE OUTLET RATE
BENCHMARK #200	2.42%	3.27% + T/A 1015.10 1,78%	T/CONC 1017.50	$Q_{DRAIN} = Restricted Drain Rate = 0.2$ $Q_{VRR} = 1.1055 - 0.206LN(A) = 1.000$ $Q_{100P} = (LESSER OF Q_{DRAIN} & Q_{VRR})^*A = 0.084$
BENCHMARK #200 ELEVATION=1014.63 BOSS NAIL/TAG SET S/S POWER POLE	+ CDS (CDS)	1017.40	[08] TOB	100-YEAR DETENTION VOLUME V _{100R} = 18985 (C)(A) = 5901
048——BFE=1012.19 ——FFE=1020.79	T/A 1018.68 RIM 1017.16	T/CONC T/A		
OUTDOOR STEPS BUILDING #4501 FI		T/A 017.50 1016.00 1017.50 1017.50		
BUILDING FFE=10 FFE: 1019.28	19.14 T/A + + + + + + + + + + + + + + + + + + +	LOT	33	+ UNDERGROUND DETENTION STORAGE PROVIDED
FFE=1020.30 © PORCH	3.4.20	1017.50 1018		ELEVATION AREA DEPTH VOLUME (FT²) (FT) (FT³) 1014.26 15.90 0.5 305.98
054 .9 FFE=1020.63 + +	+ 1019.00	T/CONC + PRO 1018.00	POSED (LDING)	HP 016.50 SHED 1013.76 14.94 0.5 527.58 1013.26 13.27 0.5 636.38 1012.76 11.26 0.5 693.30
PARCEL #11-30-101-002 + 65	-10190 (CB)	1017.50 \\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	1018.08 FG 1017.50	1012.26 9.07 0.5 711.20 1011.76 6.83 0.5 693.30 1011.26 4.64 0.5 636.35
1.62 AC.± LOT 32	EXISTING SEPTIC TANK 1017.20	1.50% T/A T/A 1017.50		/ 1010.76 2.63 0.5 527.61 / 1010.26 0.97 0.5 305.98
055		1017.50 FG 1 1017.70		ZONED: (LV) 1009.76 0.00 LAKE & VILLAGE OUTLET CONTROL STRUCTURE
000 + 100	TUCTURES AY AREA T/A 1018.50	PROPOSED S	017	$\frac{Q_{ED} \text{ ACTUAL}}{H_{ED}} = 1 \qquad (1" \text{ HOLES})$ $A_{ED} = 0.0055 \qquad \text{FT}^2$
70NFD: (LV) + 1059	X X 1019	SEPTIC TANK	× × v	$\mathbb{Q}_{\text{ED-ACTUAL}} = (A_{\text{ED}})(0.62 \times (2 \times 32.2 \times h)^{0.5}) = 0.05.$ FOR PUBLIC UTILITIES (L. 82, P. 37, O.C.R.) $\mathbb{Q}_{\text{100ALL}} = (A_{\text{ED}})(0.62 \times (2 \times 32.2 \times h)^{0.5}) = 0.05.$
ZONED: (LV) LAKE & VILLAGE SEPTIC FIELD	1019 BM	1020		$Q_{100-ACTUAL} = Q_{100P} - Q_{ED-ACTUAL} = 0.030 \text{ CFS}$ $A_{100} = Q_{100-ACTUAL} / (0.62 * (2 * 32.2 * (ELEV_{DHWL} - ELEV_{ED}))^{0.5}) =$
1 062 - 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	+	*/ **/ **/ **/ **/ **/ **/ **/ **/ **/	+1011/ +1011/ 1011/	AREA OF 1 INCH DIAMETER ORIFICE = #ORIFICES = A100 / 0.005 = 0.0 ORIFICES
Post 1" 1016 1016	+ + + 000	BENCHMARK #202 ELEVATION=1020.17 BOSS NAIL/TAG SET		OVERFLOW SPILLWAY DESIGN Design Flow Rate: Q _{100IN} = 1.94 CFS
1015 065 +	PROPOSED PROPOSED	E/S 40" WALNUT	1015	Depth of Spillway: $H_{WEIR} = 6$ INCHES Width of Spillway: $L_{WEIR} = Q_{100IN}/3.33H_{WEIR}^{3/2} =$
+101/9-	FIELD1018 PROPOSEI SEPTIC	072 + 071 -071	0 068	BASIN DESIGN SUMMARY
No. 15. 15. 15. 15. 15. 15. 15. 15. 15. 15	FIELD	* 1018° 1018	101A.1	FOREBAY SIZE REQUIRED = FOREBAY SIZE PROVIDED =
	CHAIN LINK FENCE		101h.10	BASIN SIZE REQUIRED = BASIN SIZE PROVIDED =
	X X X X X X X X X X X X X X X X X X X	1087	+	ORIFICE DESIGN SUMMARY ELEVATION # OF HOLES MET
*WEST HIGHLAND VILLAGE"	.1.46	+10° 101km	2 203	1009.76 1.0 1011.75 0.0
(L. 82, P. 37, O.C.R.) ZONED: (LV)	18 17.3 101°		101432 1014. 101h.	OVERFLOW SPILLWAY SUMMARY WIDTH OF OVERFLOW SPILLWAY =
LAKE & VILLAGE	10, 10113	1011.35 .011.18	+.01k,10	



169 | FT³

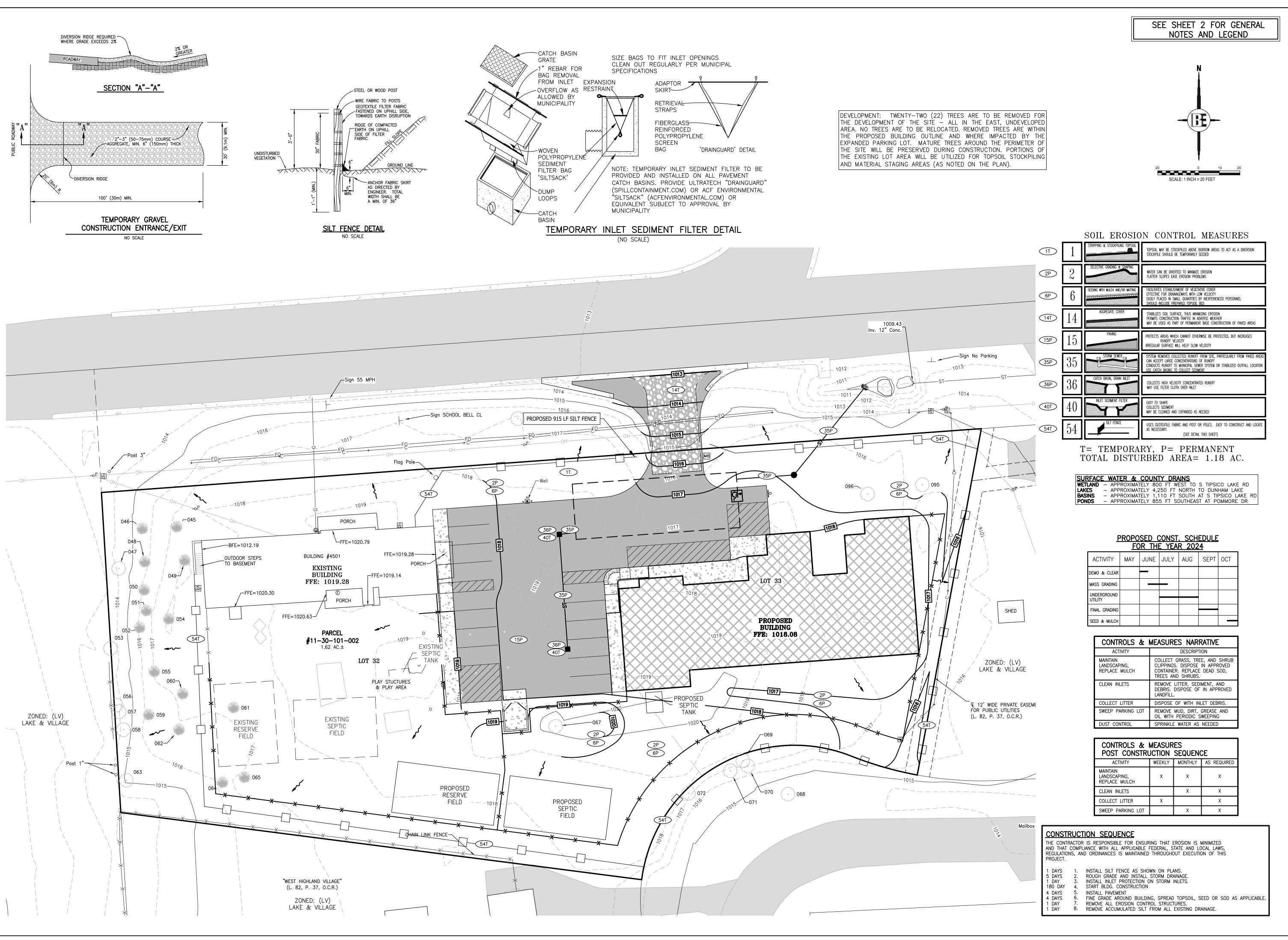
5,038 |FT³

METER OF HOLES

1

1

3993



THE L(
AS SH
GUARA!
COMPL
COMPL
LOCAT!
UTILITY
CONTR.

CONTROL SEDIMENT/ EROSION

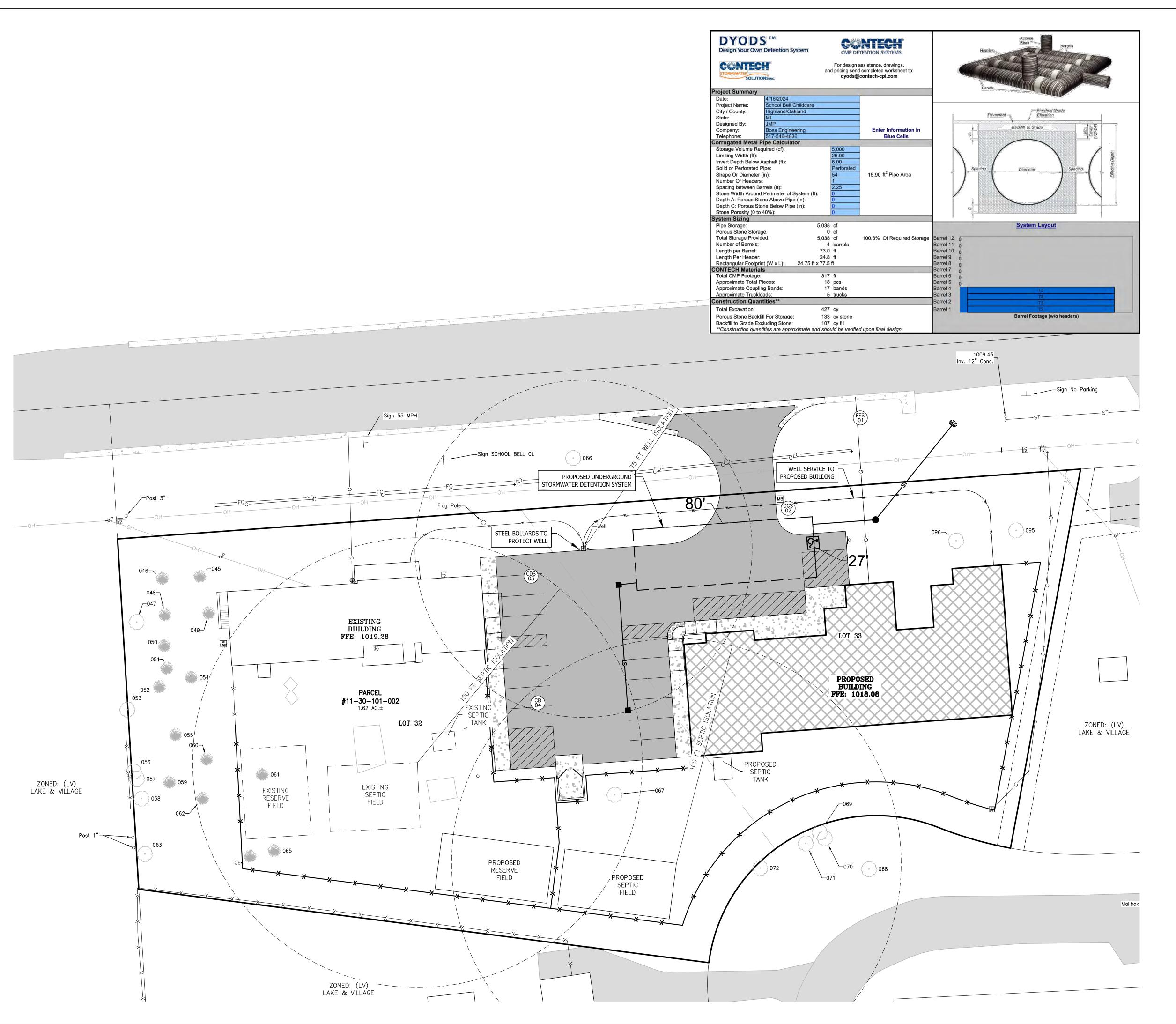
RE

BELL

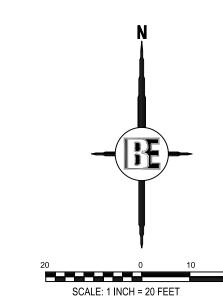
SOIL

SIGNED BY: DRAWN BY: JΡ HECKED BY:

1"=20' CALE: JOB NO: **24-048** 05/31/24

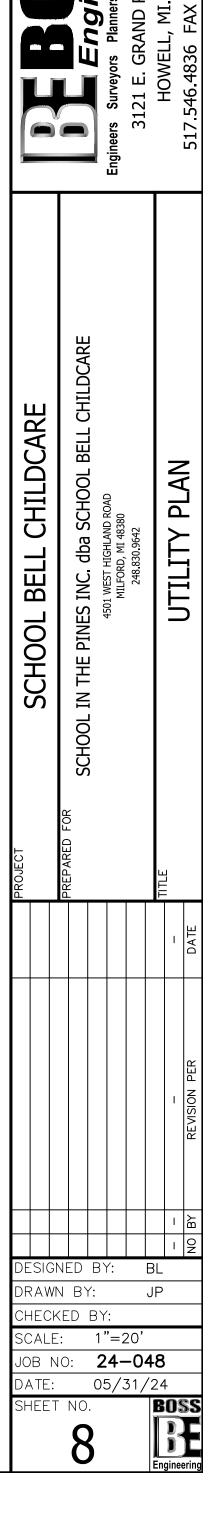


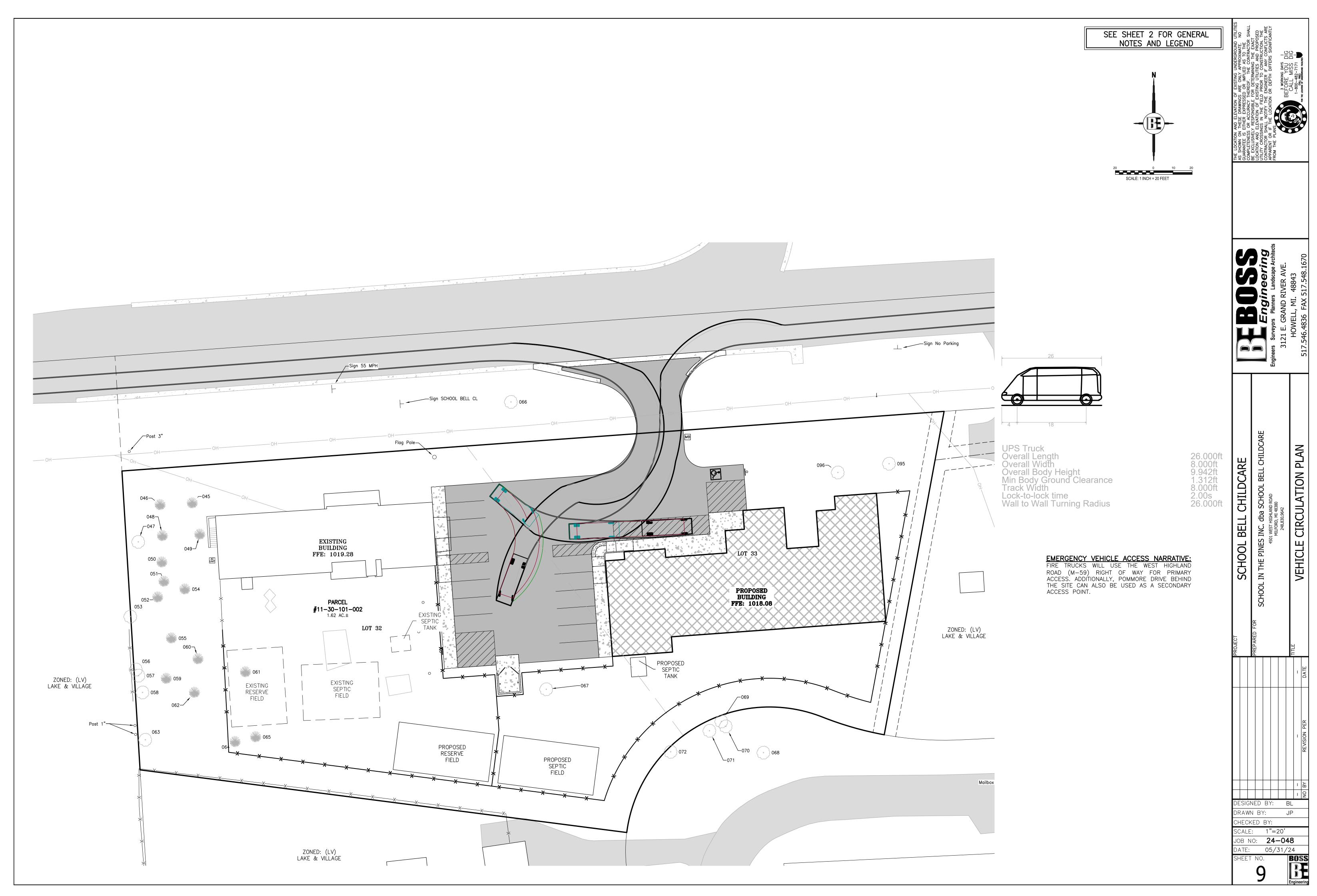
SEE SHEET 2 FOR GENERAL NOTES AND LEGEND

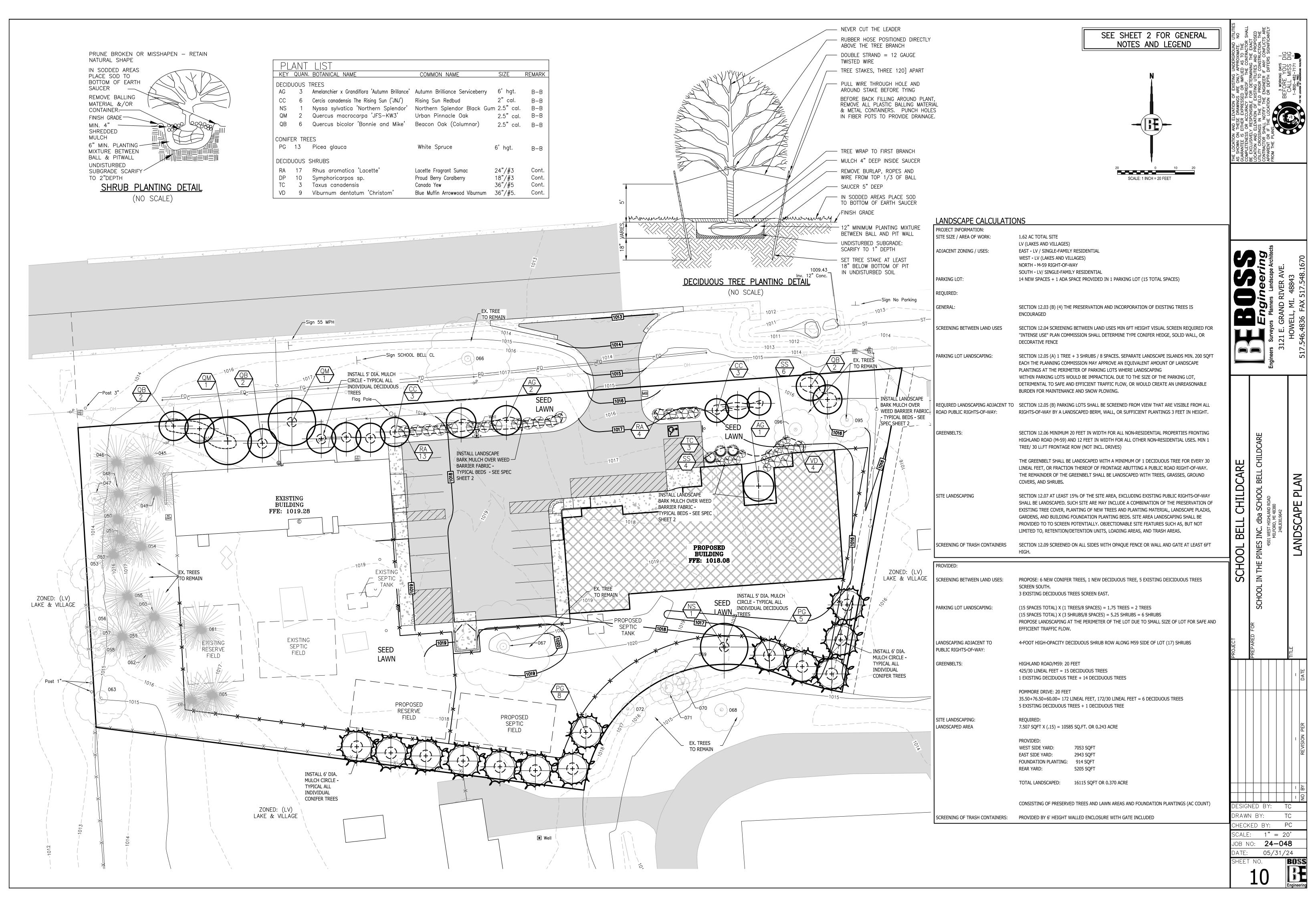


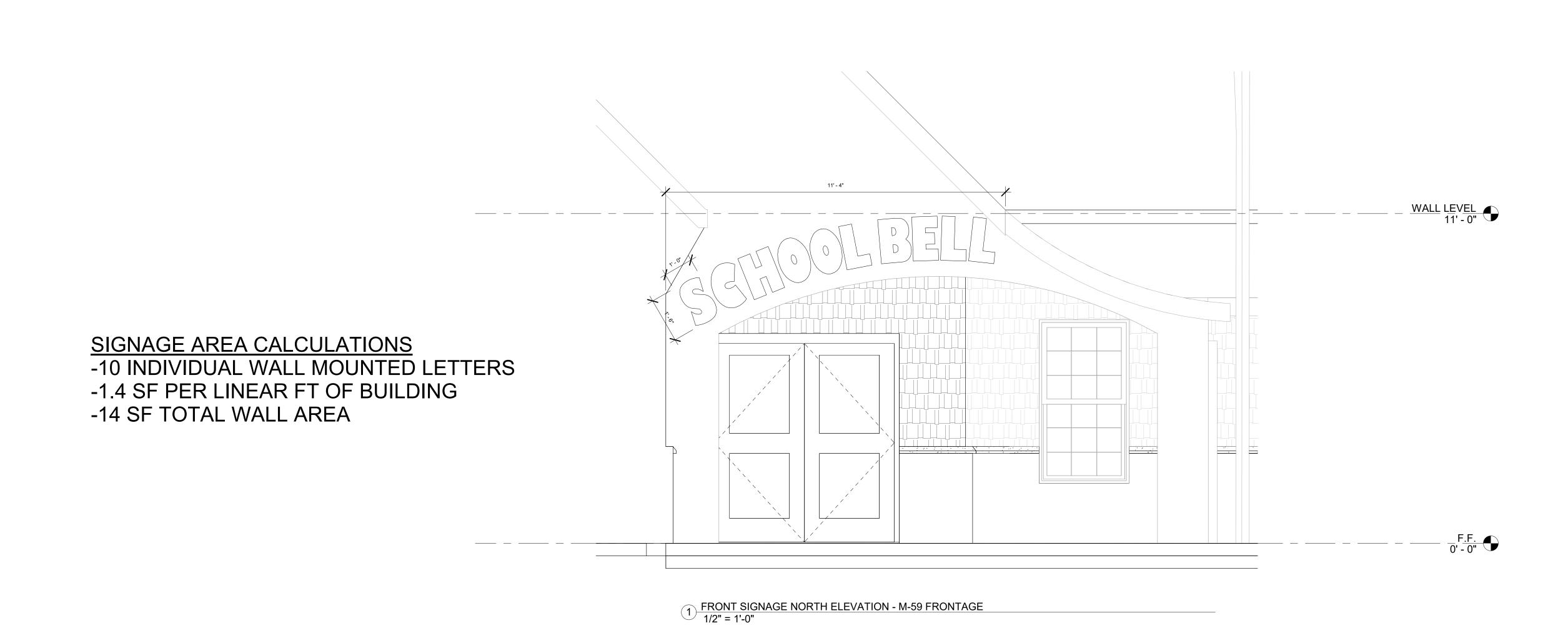
N S S S S S S S S S S S S S S S S S S S	THE LOCATION AND ELEVATION OF EXISTING UNDERCAS SHOWN ON THESE DRAWINGS ARE ONLY APPROY GUARANTEE IS EITHER EXPRESSED OR IMPLIED AS COMPLETENESS OR ACCURACY THEREOF. THE CONDER EXCLUSIVELY RESPONSIBLE FOR DETERMINING THES AN ICALITICS AND ELEVATION OF EXISTING UTILITIES AND
0 10 20	
LE: 1 INCH = 20 FEET	

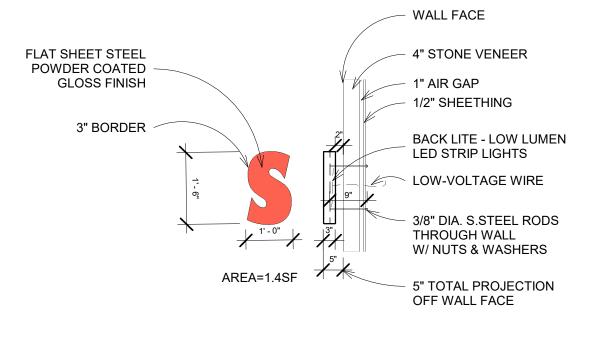
	STRUCTURE FRAMES & COVERS					
	COVER	TYPE	USE	EAST JORDAN (OR EQUAL)	TYPE OF COVER OR GRATE	
	A	МН	ALL	1040	TYPE 'B'	
	D	CB & INLET	PARKING LOTS	1040 5100	TYPE 'M1' GRATE 5105 TYPE 'M1' GRATE	
	E	CB & INLET	LAWN AREA OR DITCH	1040	TYPE '02'	
•						



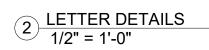








FONT: GILL SANS ULTRA BOLD CONDENSED





mkasabasic@ltu.edu

Consultant Address Address Phone Fax e-mail

No.	Description	Date

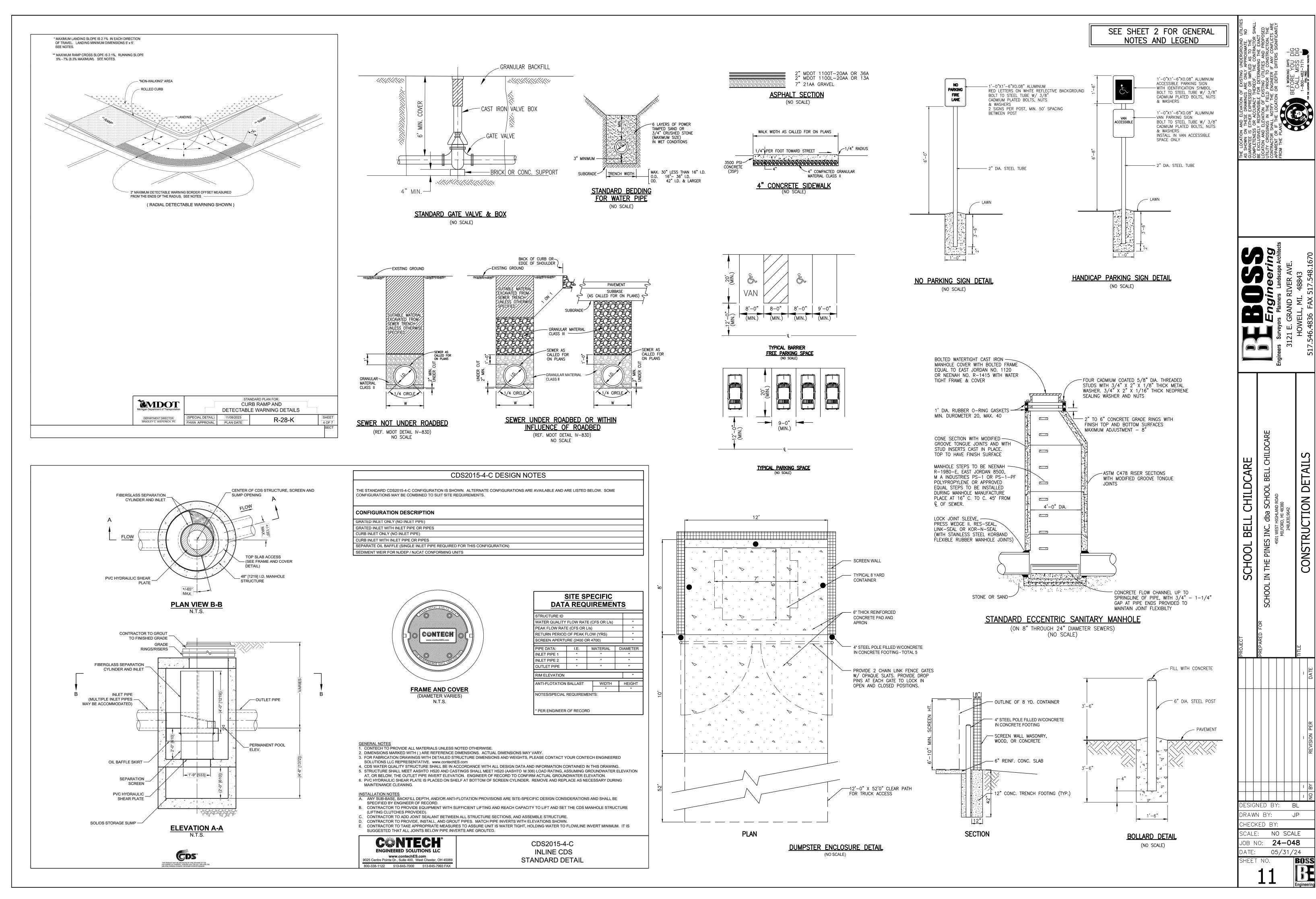
SCHOOL BELL CHILDCARE

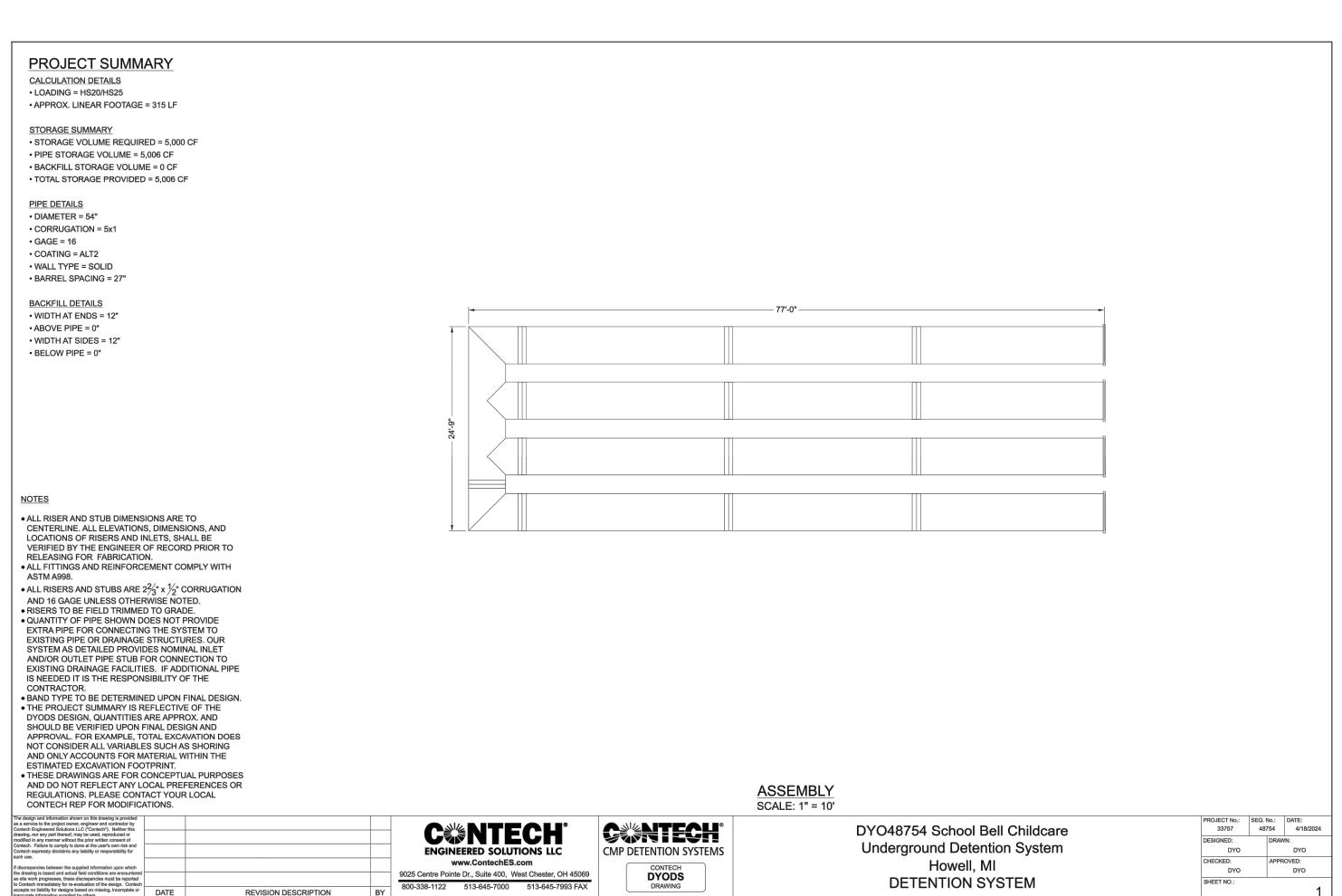
FRONT SIGNAGE

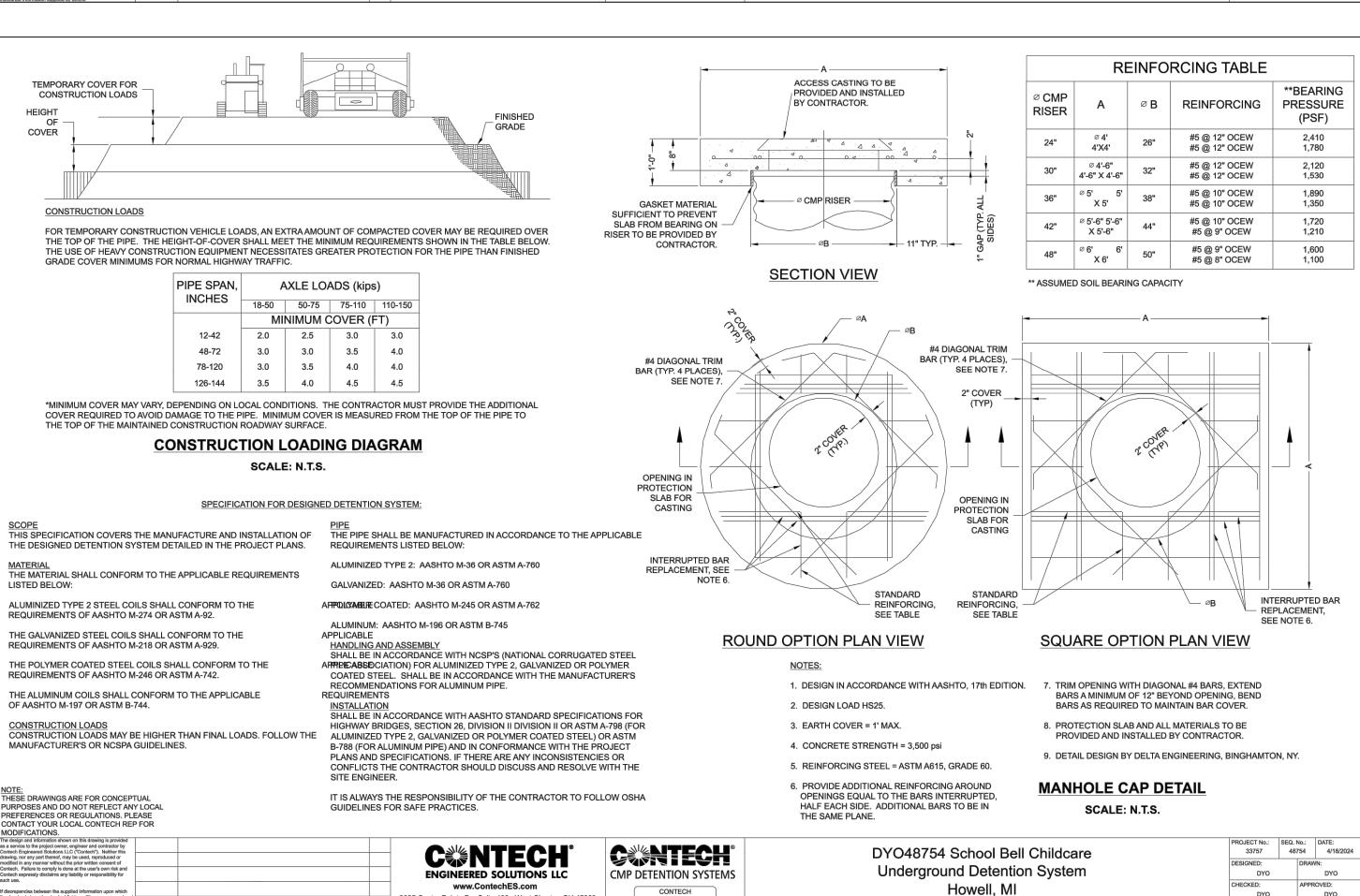
003 Project number 6/12/2024 MAK Drawn by Checked by A10

Scale

1/2" = 1'-0"





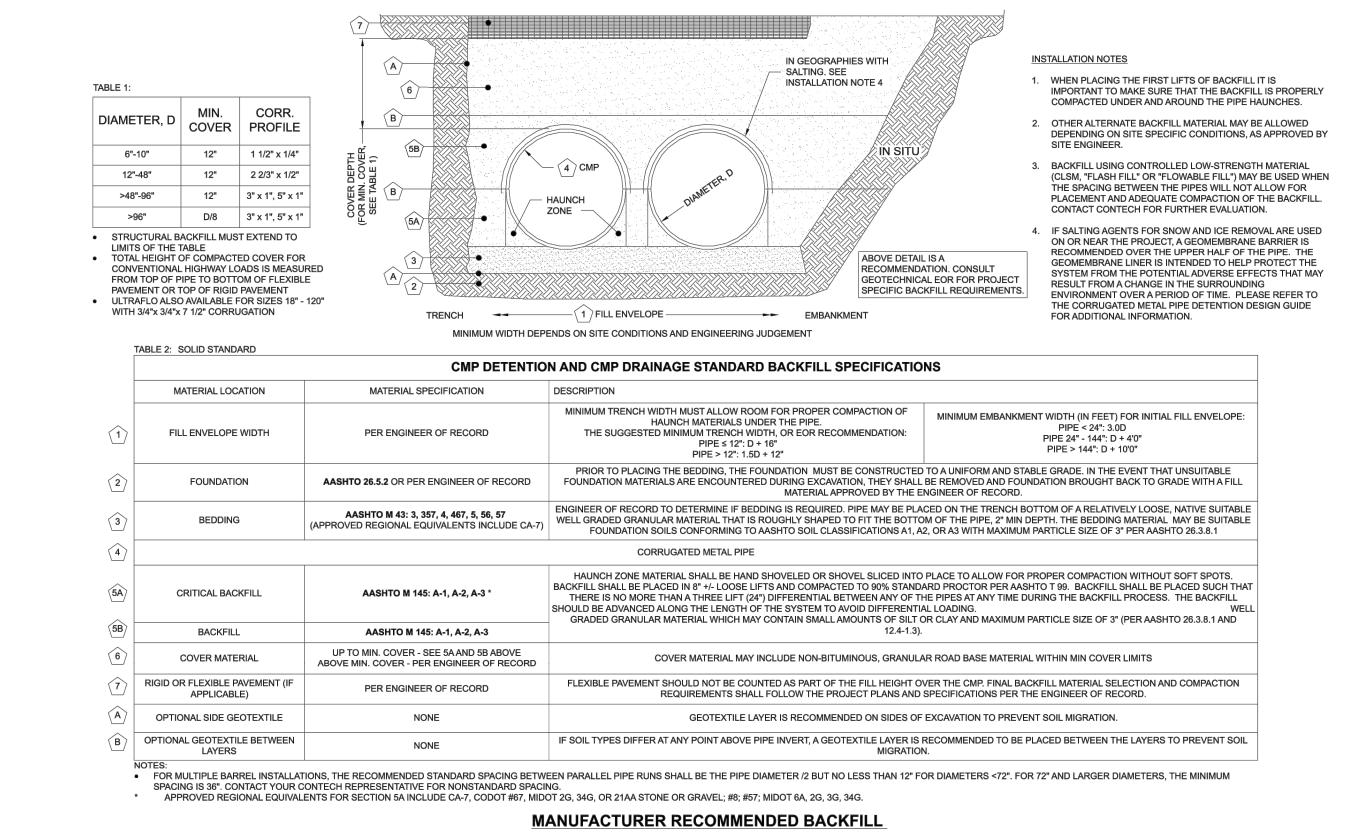


DYODS

DETENTION SYSTEM

9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069

800-338-1122 513-645-7000 513-645-7993 FAX







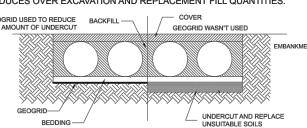
PROPER INSTALLATION OF A FLEXIBLE LINDERGROUND DETENTION SYSTEM. IF EXCAVATION IS REQUIRED. THE TRENCH WALL NEEDS TO BE CAPABLE OF WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION, CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO

DETERMINE IF ADDITIONAL MEASURES. NOT COVERED IN THIS GUIDE. ARE APPROPRIATE FOR YOUR SITE.

FOUNDATION

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION. IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS

DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE ELEVATION WITH A COMPETENT BACKFILL MATERIAL. THE STRUCTURAL FILL BEDDING - WELL GRADED MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATOR. IN SOME CASES, USING A STIFF REINFORCING GEOGRID REDUCES OVER EXCAVATION AND REPLACEMENT FILL QUANTITIES.



GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND TH CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE

GEOMEMBRANE BARRIER

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE. A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL SERVICE LIFE. THE PROJECT'S ENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING

AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE.

REVISION DESCRIPTION

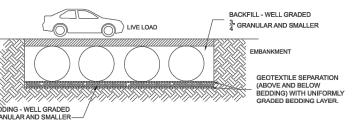
IN-SITU TRENCH WALL

BACKFILL PLACEMENT

METHODS.

SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT. PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF THE **OUTER MOST PIPES**

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN.



MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE

CONSTRUCTION LOADING TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20 LIVE LOAD, BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB, IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER

DYODS

- 8" LOOSE LIFTS

IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE

GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERE ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED, ADVANCE THE EQUIPMENT AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE, AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.

www.ContechES.com

9025 Centre Pointe Dr., Suite 400, West Chester, OH 4506

CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE WHEN FLOWABLE FILL IS USED. YOU MUST PREVENT PIPE FLOATATION

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

INSPECTION

INSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING, ANNUAL INSPECTIONS SITES WITH HIGH TRASH LOAD OR SMALL OUTLIET. CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOLLD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/ CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

MAINTENANCE

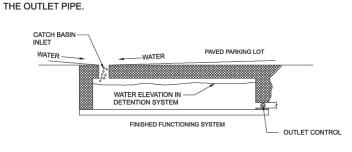
CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED

THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES. ALL PROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM. MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS

AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES

THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE NDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.



DYO48754 School Bell Childcare

Underground Detention System

Howell, MI

DETENTION SYSTEM

REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALL

CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION

POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE

MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF

TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN

ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. TH

ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER

BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING

MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT

LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP

PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM

WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING

DETERMINE THE PROPER LIFT THICKNESS

YOUR PRE-CONSTRUCTION MEETING.

ADDITIONAL CONSIDERATIONS

DESIGNED:

33757 48754 4/18/2024 DRAWN: DYO

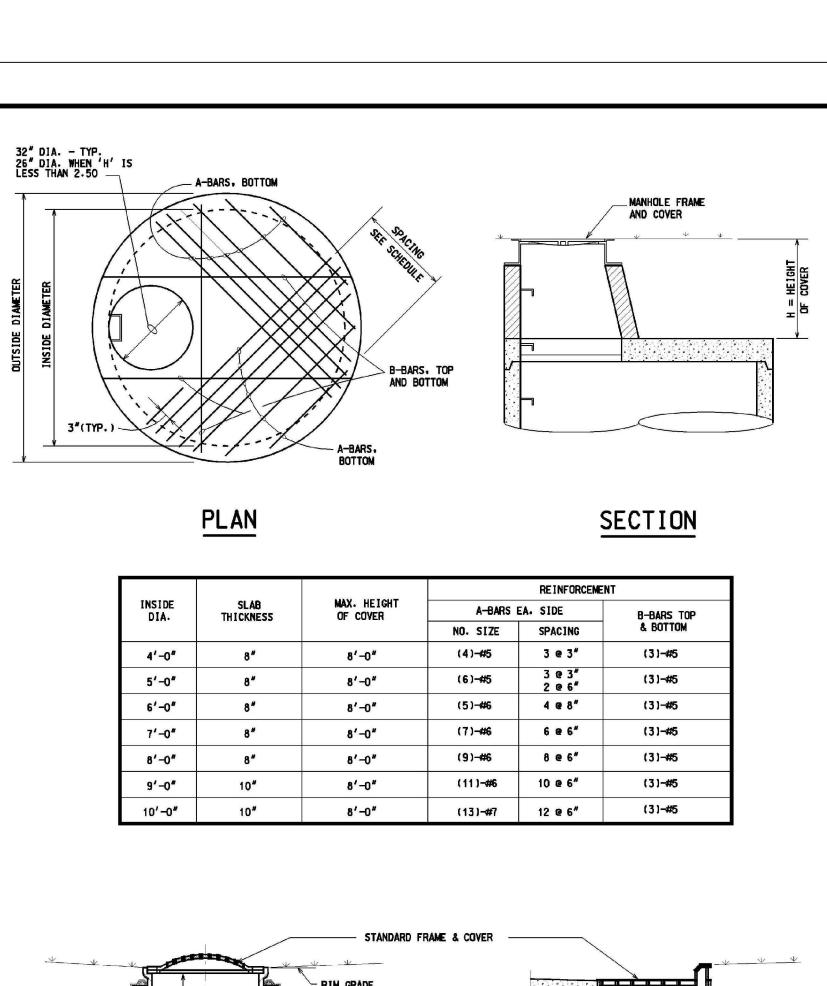
8

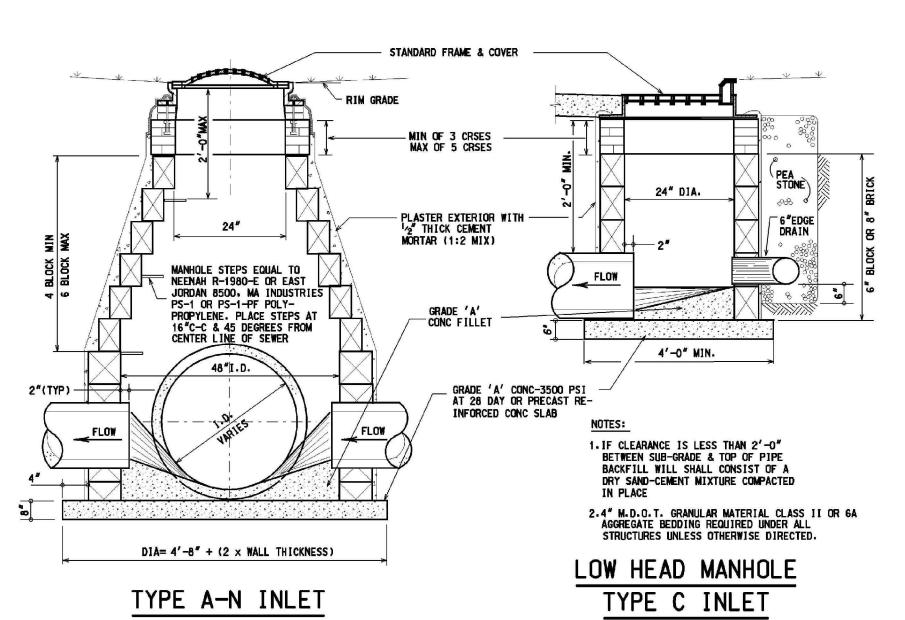
CONSTRUC

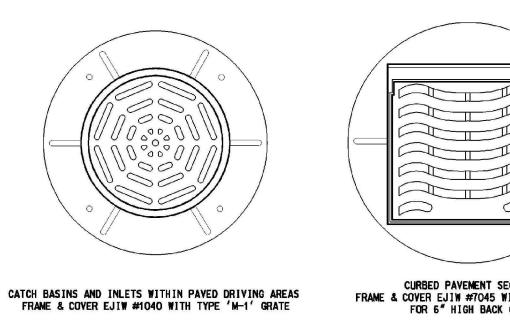
SIGNED BY: RAWN BY: HECKED BY:

NO SCALE DB NO: **24-048**

05/31/24



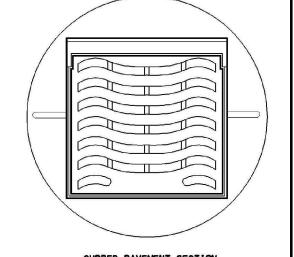


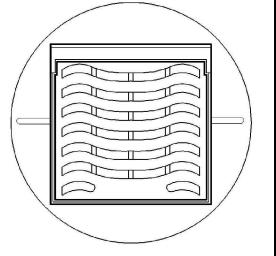


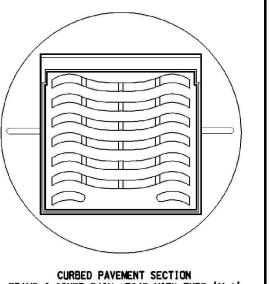
BEEHIVE FRAME & COVER EJIW #1040

WITH TYPE 02 GRATE

REAR YARD FRAME & COVER FOR 2'-0" DIA. INLET EJIW #1130 WITH TYPE 01 BEEHIVE GRATE







FRAME & COVER EJIW #7045 WITH TYPE 'M-1' FOR 6" HIGH BACK CURB

--

 \longrightarrow \subset

FRAME & COVER EJIW #7300 WITH TYPE "M"

FOR 4" MOUNTABLE CURB

STORM SEWER FRAME & COVER EJIW #1040

2" x 4" BAR RING BENT

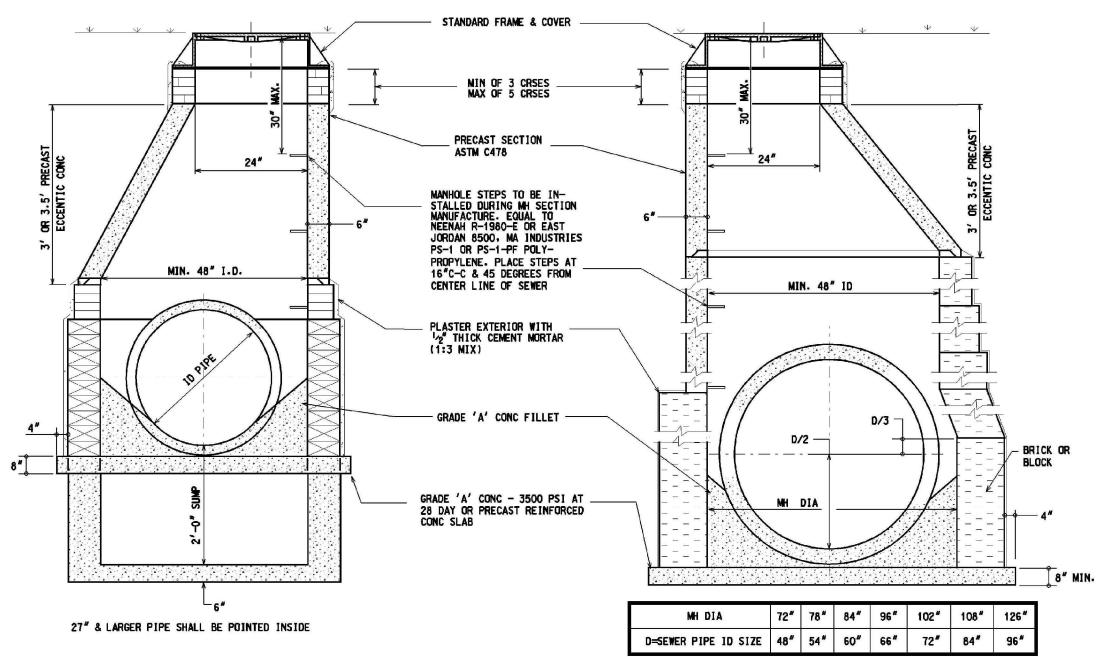
IN FIELD TO FIT PERIMETER OF END

- 8" TO 15" RIP-RAP

A MINIMUM THICKNESS

ROCK FRAGMENT

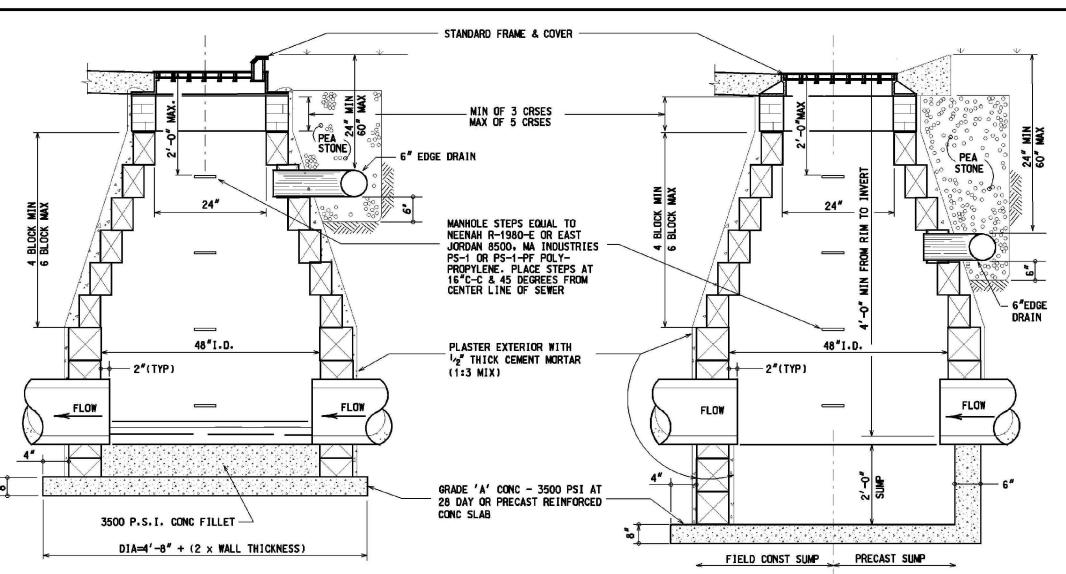
MORTARED TO A MONOLITHIC SLAB WITH

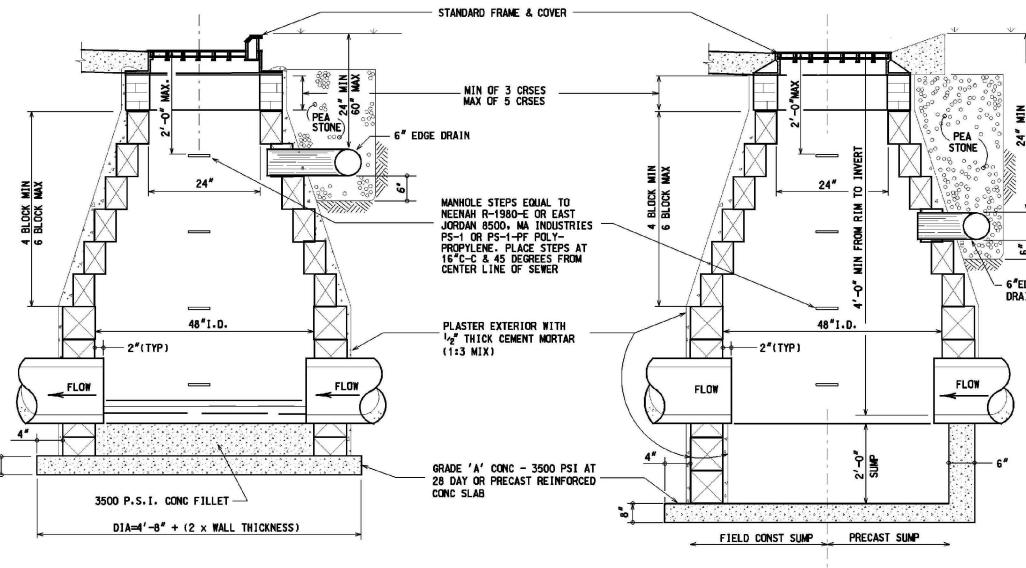




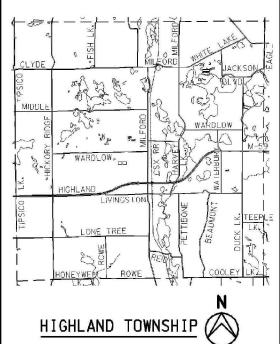
HRC







DATE DESIGNED M.P.D. DRAWN CHECKED J.B. APPROVED G.E.H.



ADDITIONS AND/OR REVISIONS

TYPE "A" INLET

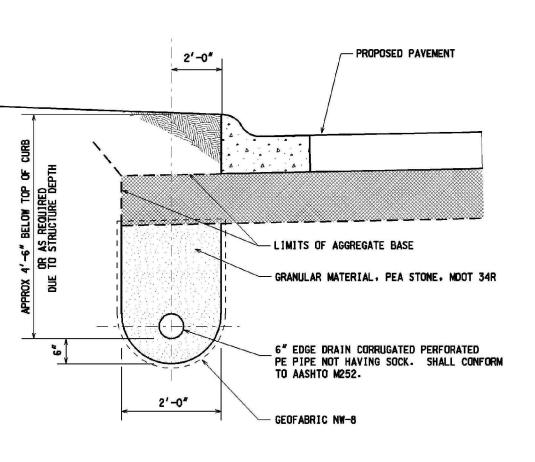
STORM MANHOLE

FOR 42" PIPE AND SMALLER

TYPE "B" CATCH BASIN



- 1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT ENGINEERING DESIGN STANDARDS AND SPECIFICATIONS OF HIGHLAND TOWNSHIP.
- 2. IT SHALL BE THE OWNER'S ENGINEER AND CONTRACTOR'S RESPONSIBILITY TO VERIFY THE EXISTENCE AND LOCATION OF ALL UNDERGROUND UTILITIES.
- 3. ALL SEWER TRENCHES UNDER THE 45 DEGREE ZONE OF INFLUENCE LINE OF EXISTING OR PROPOSED PAYEMENTS, BIKE PATHS, SIDEWALKS OR DRIVE APPROACHES SHALL BE BACKFILLED WITH MDOT CLASS II SAND COMPACTED TO AT LEAST 95% OF MAXIMUM UNIT WEIGHT.
- 4. ALL STORM SEWER SHALL BE INSTALLED ON CLASS "B" BEDDING OR BETTER.
- 5. JOINTS FOR STORM SEWER SHALL BE PREMIUM JOINTS (TONGUE AND GROOVE WITH RUBBER GASKETS).
- 6. LEAD MATERIAL SHALL BE 4" DIA. (MIN.) PVC SCHEDULE 40 OR SDR 23.5. LEAD CONNECTIONS MAY ONLY BE AT STRUCTURES.
- 7. CONTACT THE TOWNSHIP ENGINEER 48 HOURS PRIOR TO STORM SEWER INSTALLATION TO SCHEDULE OBSERVATION. FULL TIME OBSERVATION IS REQUIRED FOR ALL UNDERGROUND STORM SEWER AND LEACHING SYSTEM CONSTRUCTION. CONTACT MICHAEL DARGA WITH HUBBELL, ROTH & CLARK, INC.
- 8. BEFORE YOU DIG CALL MISS DIG AT 1-800-482-7171.
- 9. ALL MORTAR AND CONCRETE WORK SHALL BE PROTECTED FROM FREEZING (40° F. AND FALLING) FOR A MINIMUM OF 48 HOURS.
- 10. PIPE FOR STORM SEWERS WITHIN THE PUBLIC ROAD RIGHT-OF-WAY OR PRIVATE ROAD EASEMENT SHALL BE RCP. C-76. CLASS IV OR V RCP.
- 11. DOUBLE WALLED HOPE MEETING THE REQUIREMENTS OF ASTM F2306.

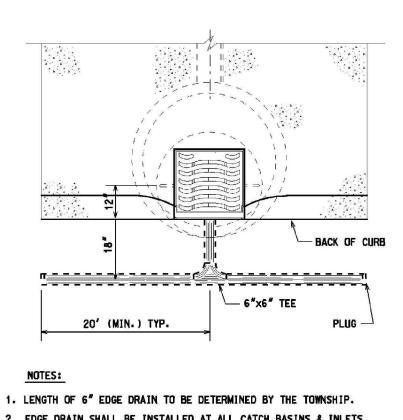


END SECTION AND BAR SCREEN DETAIL

INCLUDING RIP-RAP

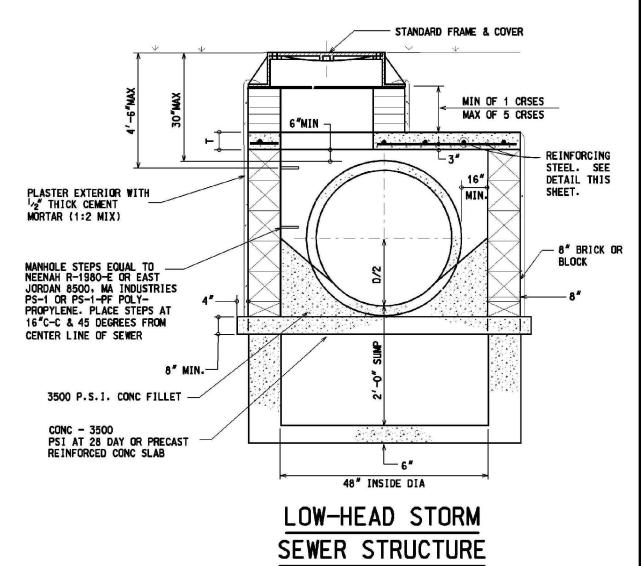
FRAMES AND COVERS

EDGE DRAIN DETAIL



2. EDGE DRAIN SHALL BE INSTALLED AT ALL CATCH BASINS & INLETS WITHIN LIMITS OF PAVEMENT OF A ROADWAY. 20' (MIN.) IN EACH DIRECTION.

6" EDGE DRAIN



STORM MANHOLE

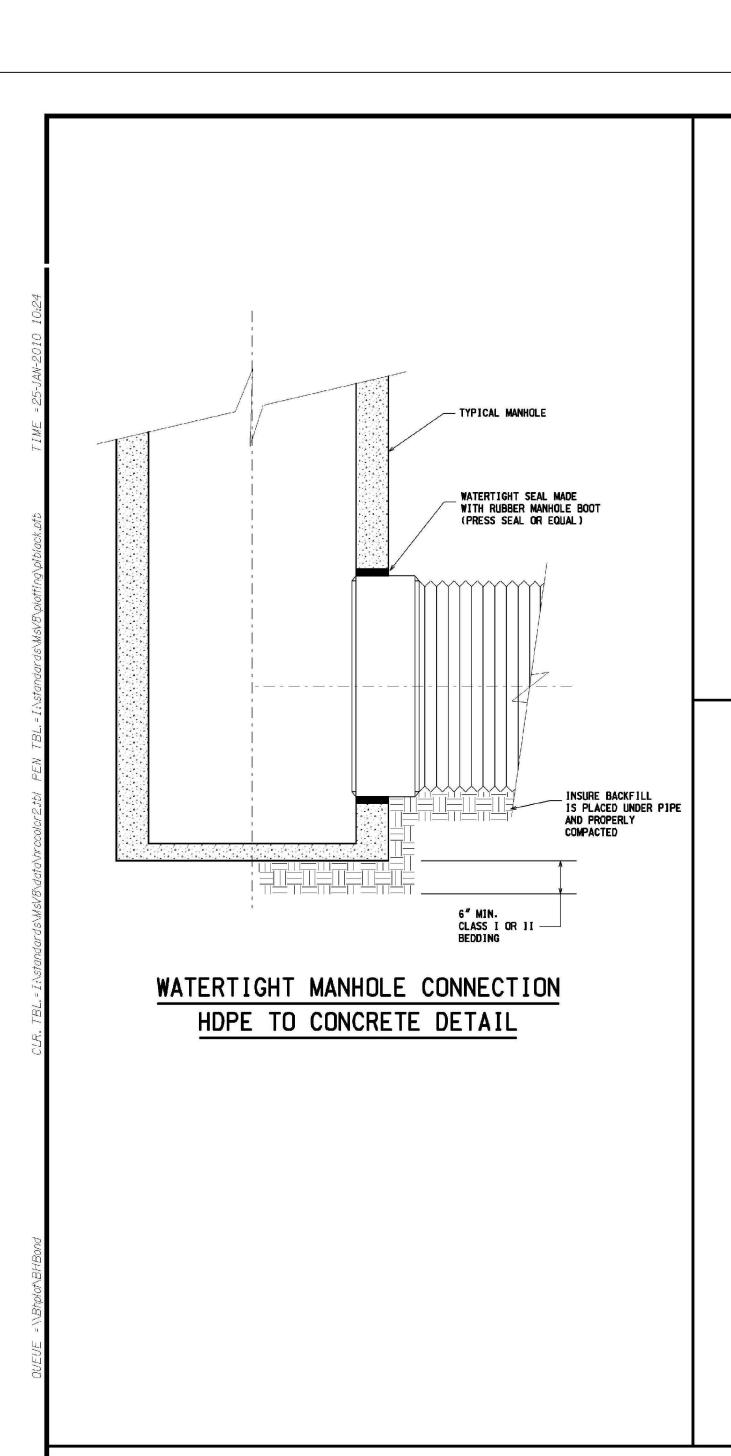
FOR 48" PIPE AND LARGER

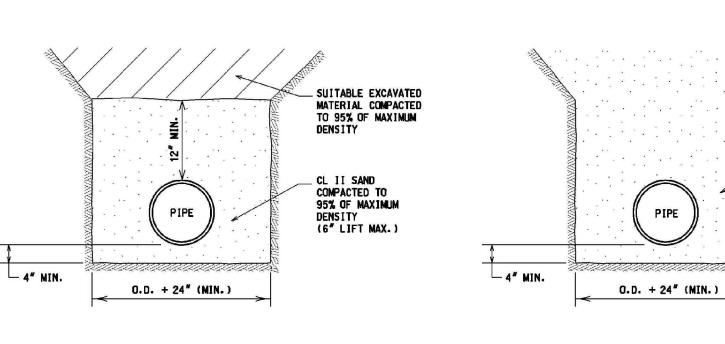
HIGHLAND TOWNSHIP **DESIGN STANDARDS**

STORM SEWER DETAILS

20050368 NONE SEPTEMBER 2005

G:\24-048\DWG\SP\24-048 SP SHEETS.dwg, 5/31/2024 9:12:59 AM, jaredp, DWG To PDF.pc3



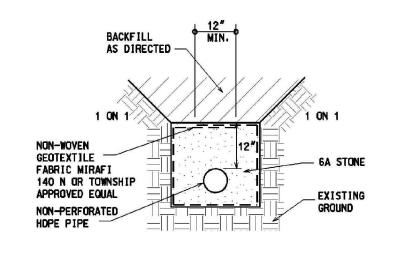


BEDDING DETAIL - TRENCH B

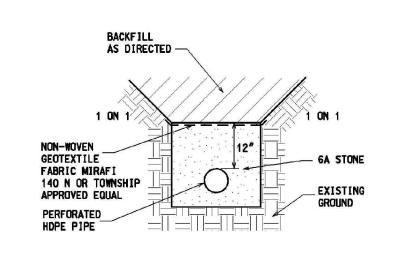
BEDDING DETAIL - TRENCH A (REQUIRED FOR INSTALLATION UNDER PAVEMENT OR

CL II SAND COMPACTED TO 95% OF MAXIMUM

DENSITY (6" LIFT MAX.)



HDPE PIPE TRENCH DETAIL FOR DETENTION SYSTEMS WITHOUT GROUNDWATER RECHARGE



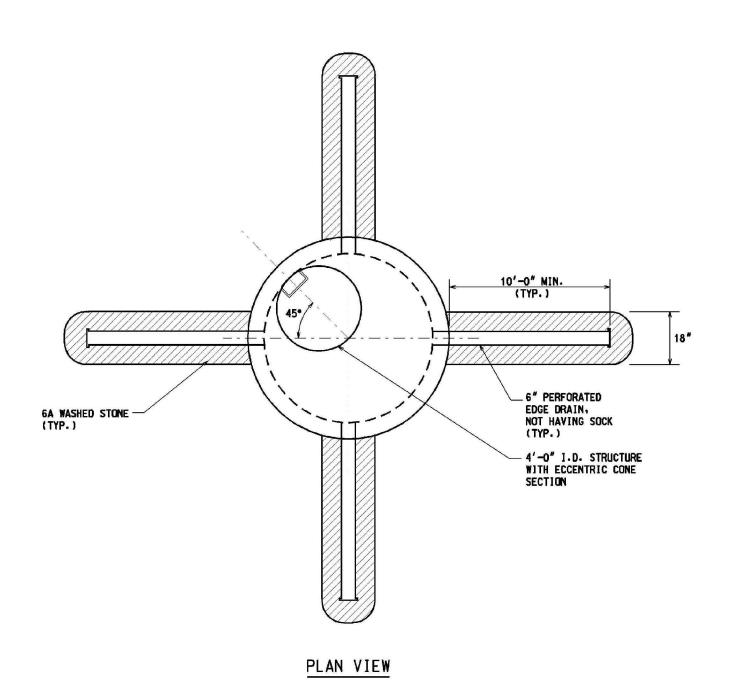
HDPE PIPE TRENCH DETAIL FOR DETENTION/RETENTION SYSTEMS WITH GROUNDWATER RECHARGE

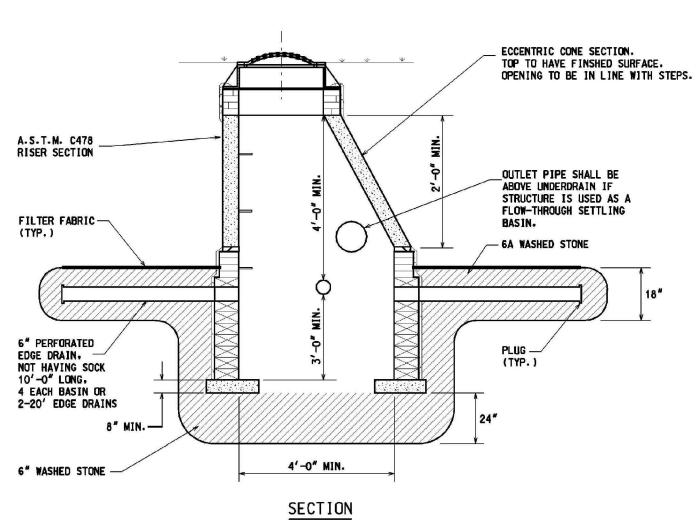


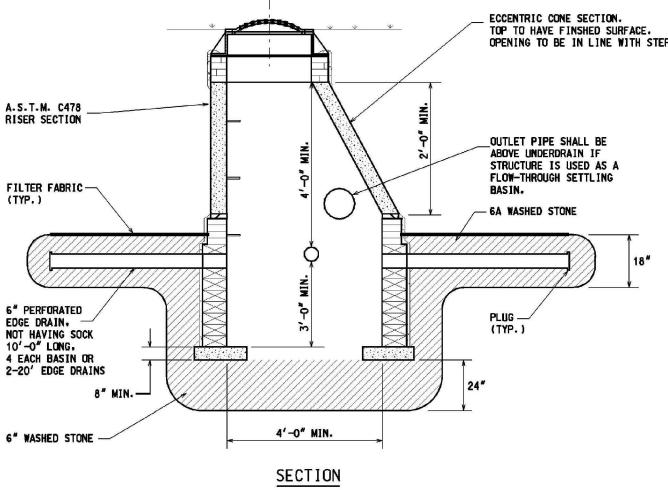
HRC

Hubbell, Roth & Clark, Inc. CONSULTING ENGINEERS 105 W. GRAND RIVER AVE.

HOWELL, MICHIGAN PHONE: (248) 454-6300 DIRECT PHONE: (517) 552-9199 FAX: (517) 552-6099 WEB SITE: http://www.hrc-engr.com

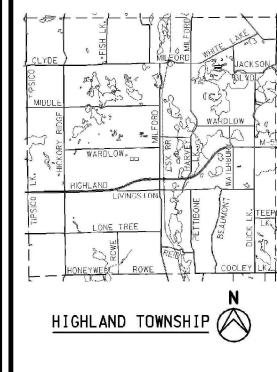






STANDARD LEACHING BASIN

ADDITIONS AND/OR REVISIONS DESIGNED M.P.D. DRAWN T.E.W. CHECKED J.B. APPROVED G.E.H.

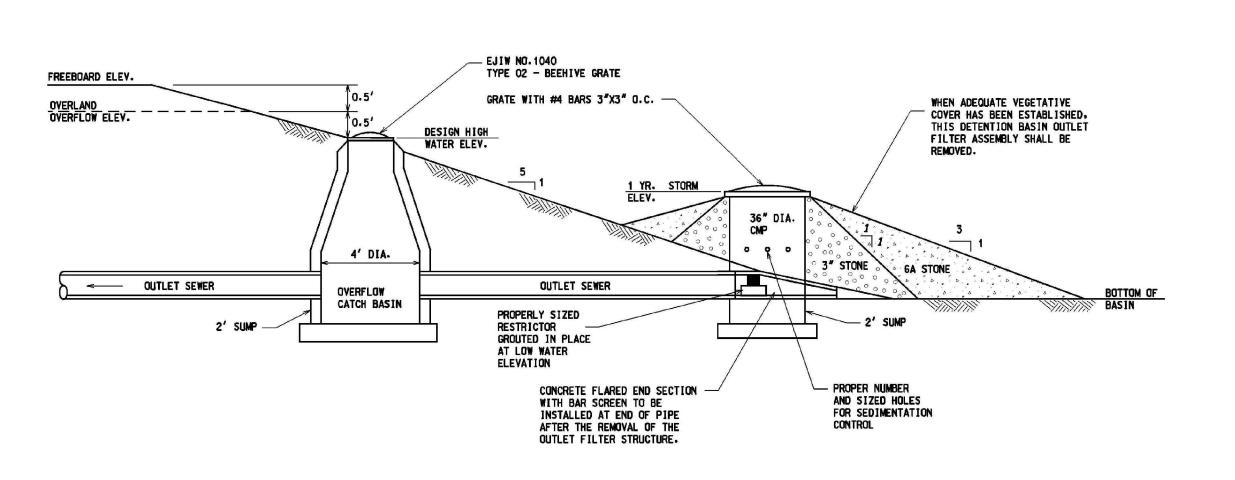


HIGHLAND TOWNSHIP DESIGN STANDARDS

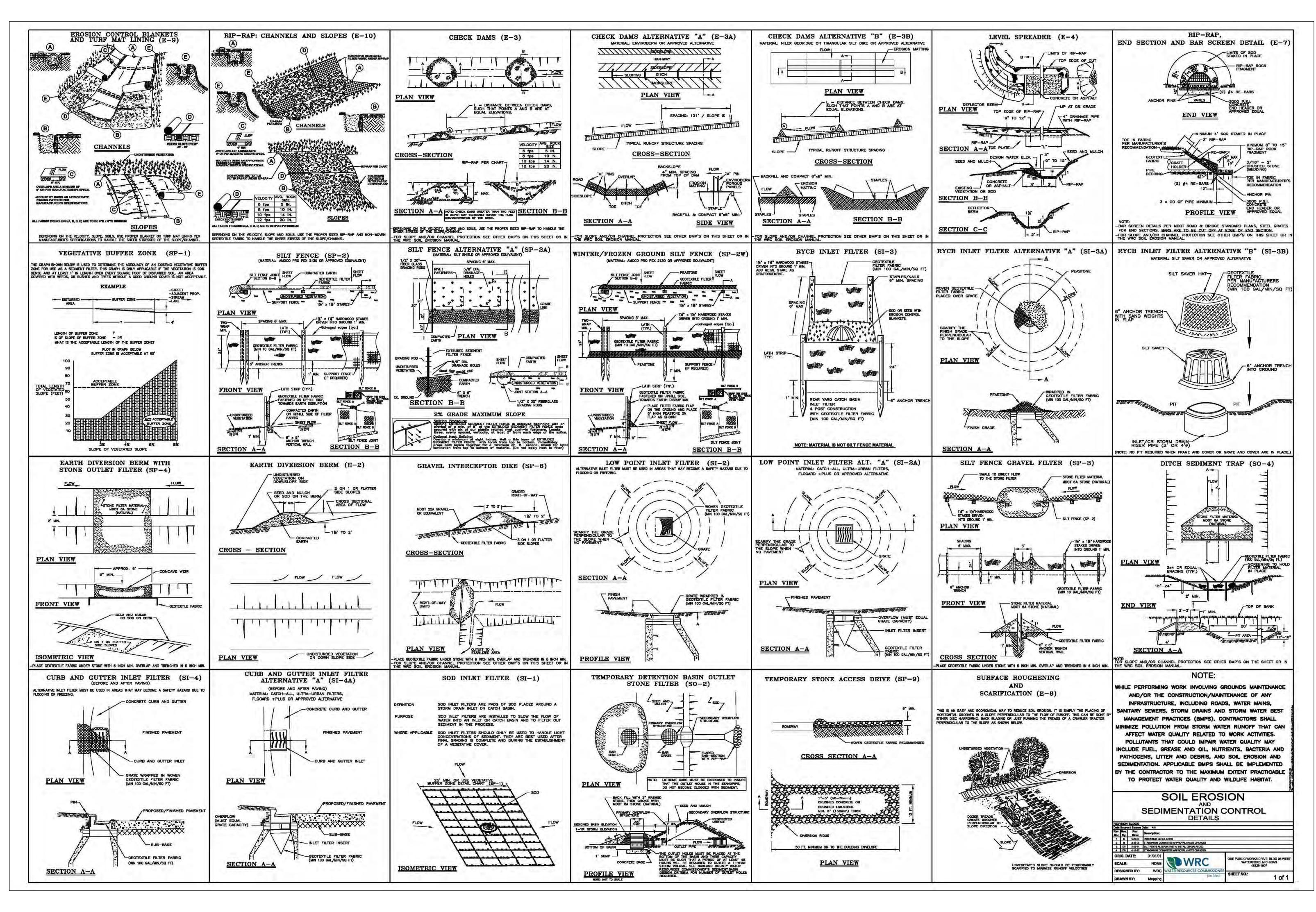
STORM SEWER DETAILS

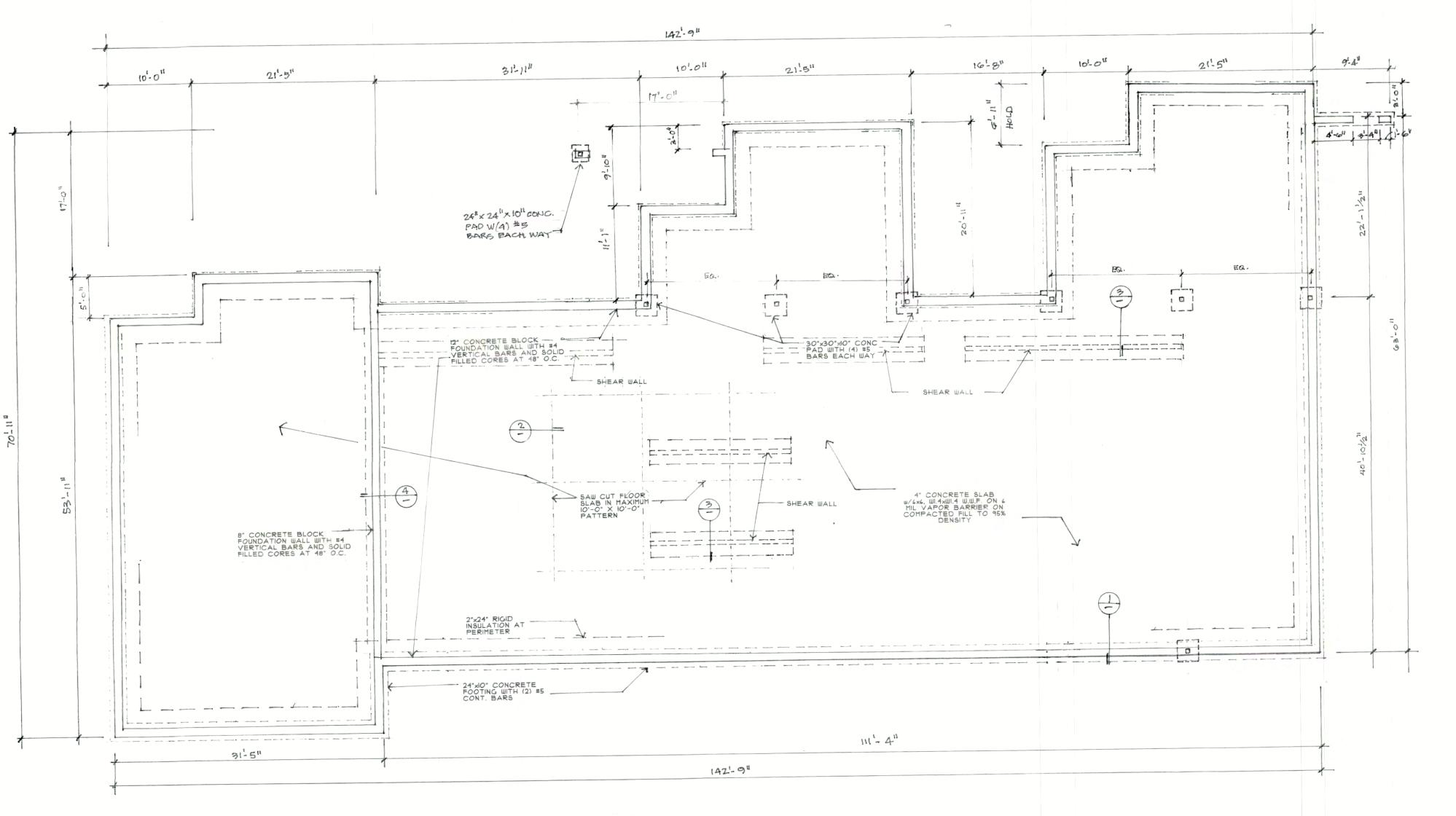
	HRC JOB NO. 20050368	SCALE	NONE	
	DATE SEPTEMBER 2005	SHEET NO.	2	OF 2
© 2005 E	lubbell, Roth and C ark,	Inc. All	Rights	Reserved

- 1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT ENGINEERING DESIGN STANDARDS AND SPECIFICATIONS OF HIGHLAND TOWNSHIP.
- 2. IT SHALL BE THE OWNER'S ENGINEER AND CONTRACTOR'S RESPONSIBILITY TO VERIFY THE EXISTENCE AND LOCATION OF ALL UNDERGROUND UTILITIES.
- 3. ALL SEWER TRENCHES UNDER THE 45 DEGREE ZONE OF INFLUENCE LINE OF EXISTING OR PROPOSED PAVEMENTS, BIKE PATHS, SIDEWALKS OR DRIVE APPROACHES SHALL BE BACKFILLED WITH MOOT CLASS II SAND COMPACTED TO AT LEAST 95% OF MAXIMUM UNIT WEIGHT.
- 4. ALL STORM SEWER SHALL BE INSTALLED ON CLASS "B" BEDDING OR BETTER.
- 5. JOINTS FOR STORM SEWER SHALL BE PREMIUM JOINTS (TONGUE AND GROOVE WITH RUBBER GASKETS).
- 6. LEAD MATERIAL SHALL BE 4" DIA. (MIN.) PVC SCHEDULE 40 OR SDR 23.5. LEAD CONNECTIONS MAY ONLY BE AT STRUCTURES.
- 7. CONTACT THE TOWNSHIP ENGINEER 48 HOURS PRIOR TO STORM SEWER INSTALLATION TO SCHEDULE OBSERVATION. FULL TIME OBSERVATION IS REQUIRED FOR ALL UNDERGROUND STORM SEWER AND LEACHING SYSTEM CONSTRUCTION. CONTACT MICHAEL DARGA WITH HUBBELL, ROTH & CLARK, INC.
- 8. BEFORE YOU DIG CALL MISS DIG AT 1-800-482-7171.
- 9. ALL MORTAR AND CONCRETE WORK SHALL BE PROTECTED FROM FREEZING (40° F. AND FALLING) FOR A MINIMUM OF 48 HOURS.
- 10. PIPE FOR STORM SEWERS WITHIN THE PUBLIC ROAD RIGHT-OF-WAY OR PRIVATE ROAD EASEMENT SHALL BE RCP, C-76, CLASS IV OR V RCP.
- 11. DOUBLE WALLED HOPE MEETING THE REQUIREMENTS OF ASTM F2306.

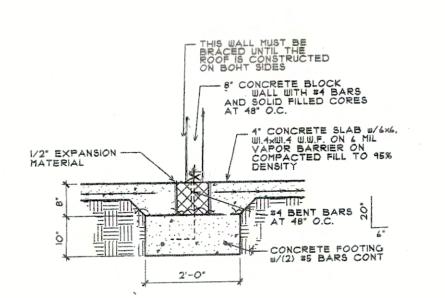


DETENTION POND OUTLET STRUCTURE DETAIL

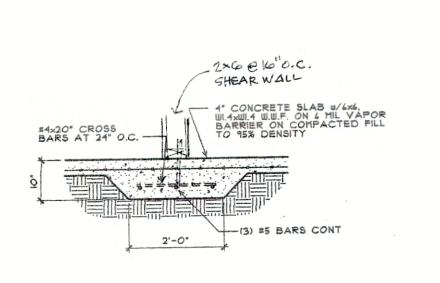




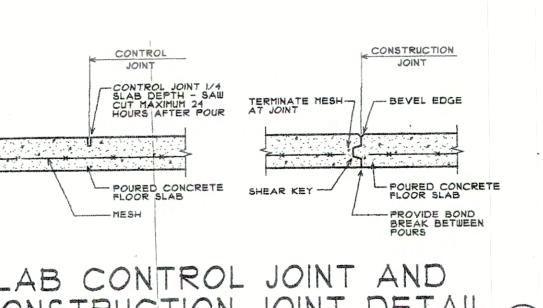




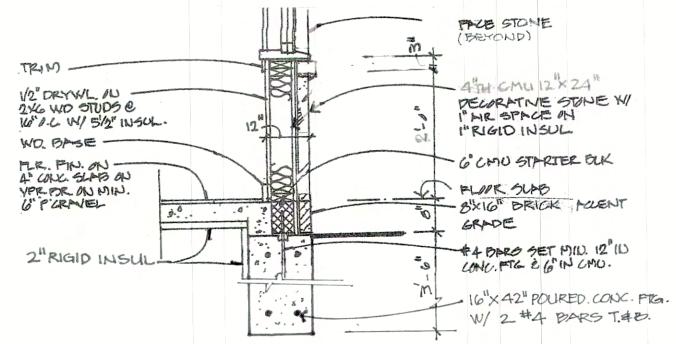




FOUNDATION DETAIL 3



SLAB CONTROL JOINT AND CONSTRUCTION JOINT DETAIL SCALE: NONE



DET, @ ALT. TRENCH FTG. SCALE 1/2" = 1:0"

CONCRETE NOTES:

ALL READY MIX CONCRETE SHALL CONFORM TO A.S.T.M. C 94. 2. MEASURING, MIXING, TRANSPORTING, AND PLACING OF ALL CONCRETE SHALL COMPLY W/ ACI 304R-00

3. ALL REINFORCING BARS, DOWELS, ANCHOR BOLTS AND ANY INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING OF CONCRETE.

4. AIR ENTRAINED CONCRETE SHALL BE USED FOR ALL SIDEWALKS, PAVING, PLATFORMS, CURBS, AND ALL CONCRETE ELEMENTS EXPOSED TO THE WEATHER.

5. ALL CONCRETE SHALL DEVELOP THE FOLLOWING COMPRESSIVE STRENGTHS AT 28 DAYS (U.N.O.)

INTERIOR CONCRETE SLABS EXTERIOR CONCRETE SLABS FOOTINGS AND FOUNDATION WALLS

LOCATE ALL SLEEVES, OPENINGS, EMBEDDED ITEMS, ETC., WHICH ARE INDICATED ON ALL DESIGN DRAWINGS. CHECK WITH ALL TRADES TO VERIFY THAT ALL SLEEVES, OPENINGS, AND EMBEDDED ITEMS ARE IN PLACE AND LOCATED CORRECTLY PRIOR TO PLACING THE ADJACENT CONCRETE. THE SURFACE OF ALL CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE ALL DUST, CHIPS, OR OTHER FOREIGN MATTER PRIOR TO PLACING THE ADJACENT CONCRETE.

8. ALL SLABS SHALL BE CURED AND SEALED TO MINIMIZE SHRINKAGE CRACKING.

O. PIPE MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES, BUT SHALL NOT BE EMBEDDED THEREIN. SLEEVES SHALL BE WRAPPED WITH EXPANSION JOINT FILLER MATERIAL TO ALLOW CONCRETE TO CURE WITHOUT RESTRAINT. PIPES OR CONDUITS EXCEEDING ONE THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND/OR ELECTRICAL DRAWINGS FOR LOCATIONS OF SLEEVES, ACCESSORIES, ETC.

DURING COLD WEATHER MONTHS, ALL CONCRETE SHALL COMPLY W/ACI COLD WEATHER CONCRETING, ACI 304R-88. PER ACI, COLD WEATHER IS DEFINED AS A PERIOD WHEN, FOR MORE THAN (3) CONSECUTIVE DAYS, THE FOLLOWING CONDITIONS MUST EXIST:

1) THE AVERAGE DAILY AIR TEMP IS LESS THAN 40 F, AND,

2) THE AIR TEMP. IS NOT GREATER THAN 50 F FOR MORE THAN 50 F THAN ONE-HALF OF ANY (24)-HR PERIOD.

13. DURING WARM WEATHER MONTHS, ALL CONCRETE SHALL COMPLY W/ ACI COLD WEATHER CONCRETING, ACI 305R-99

GENERAL NOTES:

FASTENERS AND CONNECTORS, INCLUDING NUTS AND WASHERS, IN CONTACT WITH PRESERVATIVE TREATED WOOD SHALL BE OF HOT-DIPPED. ZINC COATED GLAVANIZED STEEL

2. CONTINUOUS HORIZONTAL REROD LAP TO BE A MINIMUM OF 24"

SOIL BEARING CAPACITY: 1000 PSF (VERIFY)

Petkoski Architects

form | space | order

41 Shoreham Grosse Pointe Shores, MI 48236 tel: 313-930-2030

project

SCHOOL BELL CHILDCARE 4501 W. HIGHLAND ROAD MILFORD, MI 48380

owner

> SCHOOL IN THE PINES INC.

sheet FOUNDATION

PLAN

The ideas and design concepts expressed herein and the graphically displayed arrangement of the components represented by this drawing have been developed for the exclusive use of the specified project and are the sole prperty of the architect. Any conveyence of disclosure of the ideas or design concepts or use of any graphically displayed arrangements of their components shall be at the discretion of and only through the expressed written consent of the Pethosld Architects, L.L.C. © PETKOSIO ARCHTECTS, LLC

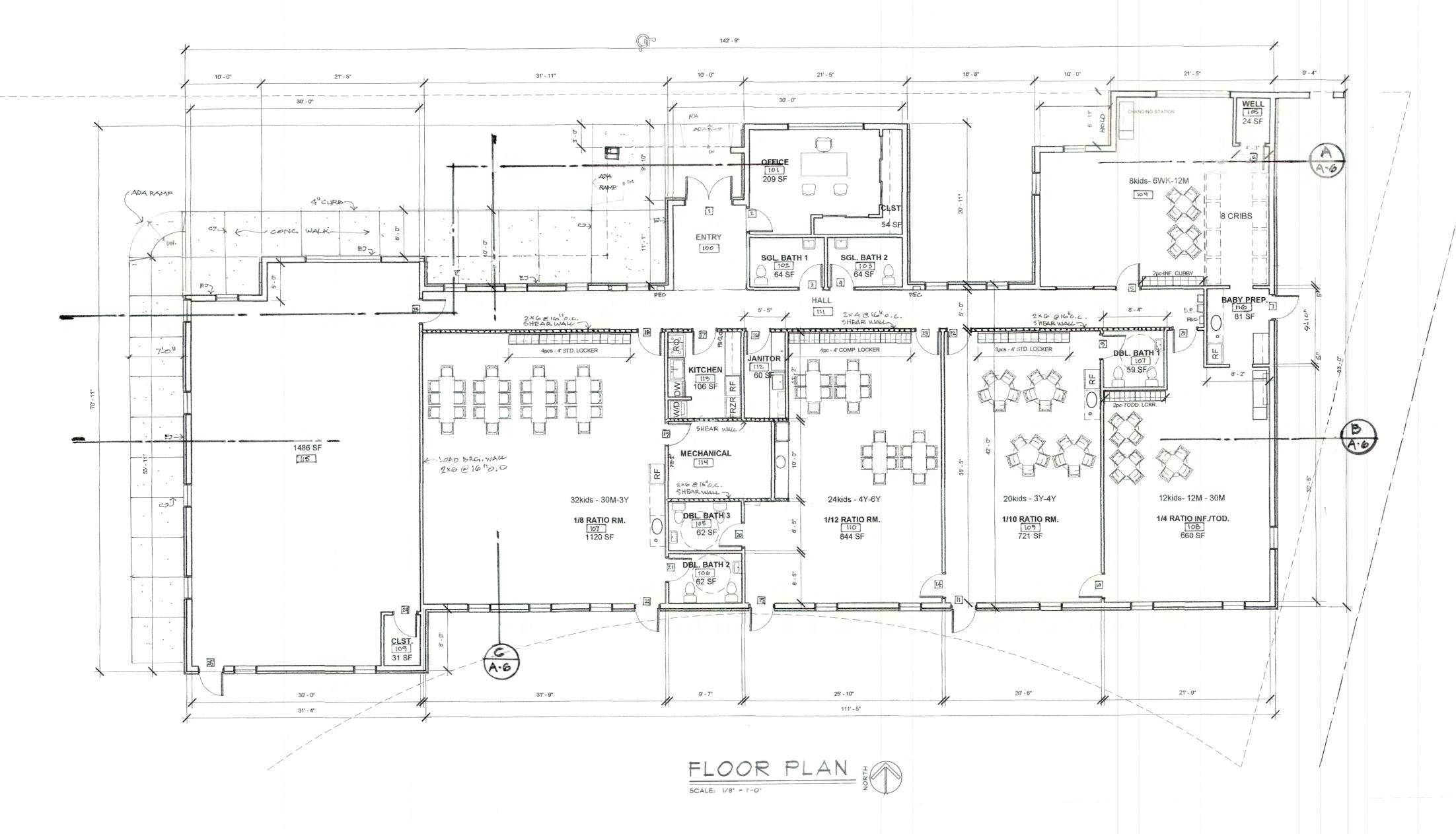
JN/BG

5-18-2024 AS NOTED

24-010

A-1





FIRE RATING NOTES: 3. ALL PENETRATIONS SHALL BE SEALED TIGHT WITH AN APPROVED FIRE CAULKING. 4. PROVIDE FIRE DAMPERS AND SMOKE DAMPERS PER MECHANICAL PLAN WHERE INDICATED 5. ALL FIRE RATED AND SMOKE BARRIER WALLS WILL BE MARKED AND LABELED ABOVE THE CEILING AND OR ATTIC, SPACING 30± FOOT INTERVALS AND IS± FEET FROM EACH END OF THE WALL, LETTERING HEIGHT TO BE 3 MINIMUM AND WIDTH OF 3/8 MIN. SUGGESTED WORDING FIRE AND/OR SMOKE BARRIER & PROTECT ALL OPENINGS - BOTTOM CHORD OF PRE-ENGINEERED ROOF TRUSS FIRE ASSEMBLY UL U419 RESILIENT METAL CHANNELS -----3 1/2", 25 GAUGE STEEL STUDS AT 16" O.C. 5/8" TYPE "C" GYPSUM BOARD-5/8" TYPE "X" GYPSUM BOARD NON-HAZARDOUS HAZARDOUS ROOM RATED CEILING/ROOF ASSEMBLY - I HOUR

FIRE EXTINGUISHER LEGEND

FEC 10# ABC FIRE EXTINGUISHER IN RECESSED CABINET

HAZARDOUS ROOM CEILING

FE-I 5# ABC FIRE EXTINGUISHER (MECHANICAL ROOM ONLY) FE-2 'K' STYLE FIRE EXTINGUISHER (KITCHEN)

Petkoski Architects

41 Shoreham Grosse Pointe Shores, MI 48236 tel: 313-930-2030 project

SCHOOL BELL CHILDCARE 4501 W. HIGHLAND ROAD MILFORD, MI 48380

owner

SCHOOL IN THE PINES INC.

sheet

FLOOR PLAN

The ideas and design concepts expressed herein and the graphically displayed arrangement of the components represented by this drawing have been developed for the exclusive use of the specified project and are the sole prperty of the architect. Any conveyence of disclosure of the ideas or design concepts or use of any graphically displayed arrangements of their components shall be at the discretion of and only through the expressed written consent of the Petitoski Architects, L.L.C. 6 PETICOSII ARCHITECIS, LLC

5-18-2024 AS NOTED



Petkoski Architects

form | space | order

41 Shoreham Grosse Pointe Shores, MI 48236 tel: 313-930-2030

tel: 313-930-2030 project

SCHOOL BELL
CHILDCARE
4501 W.
HIGHLAND ROAD
MILFORD, MI 48380

owner

SCHOOL IN THE PINES INC.

sheet

ELEVATIONS

The ideas and design concepts expressed herein and the graphically displayed arrangement of the components represented by this drawing have been developed for the exclusive use of the specified project and are the sole prperty of the architect. Any conveyence of disclosure of the ideas or design concepts or use of any graphically displayed arrangements of their components shall be at the discretion of and only through the expressed written consent of the Petkoski Architects, L.L.C.

issue / revision

Drawn by JN/BG

Date 5-18-2024

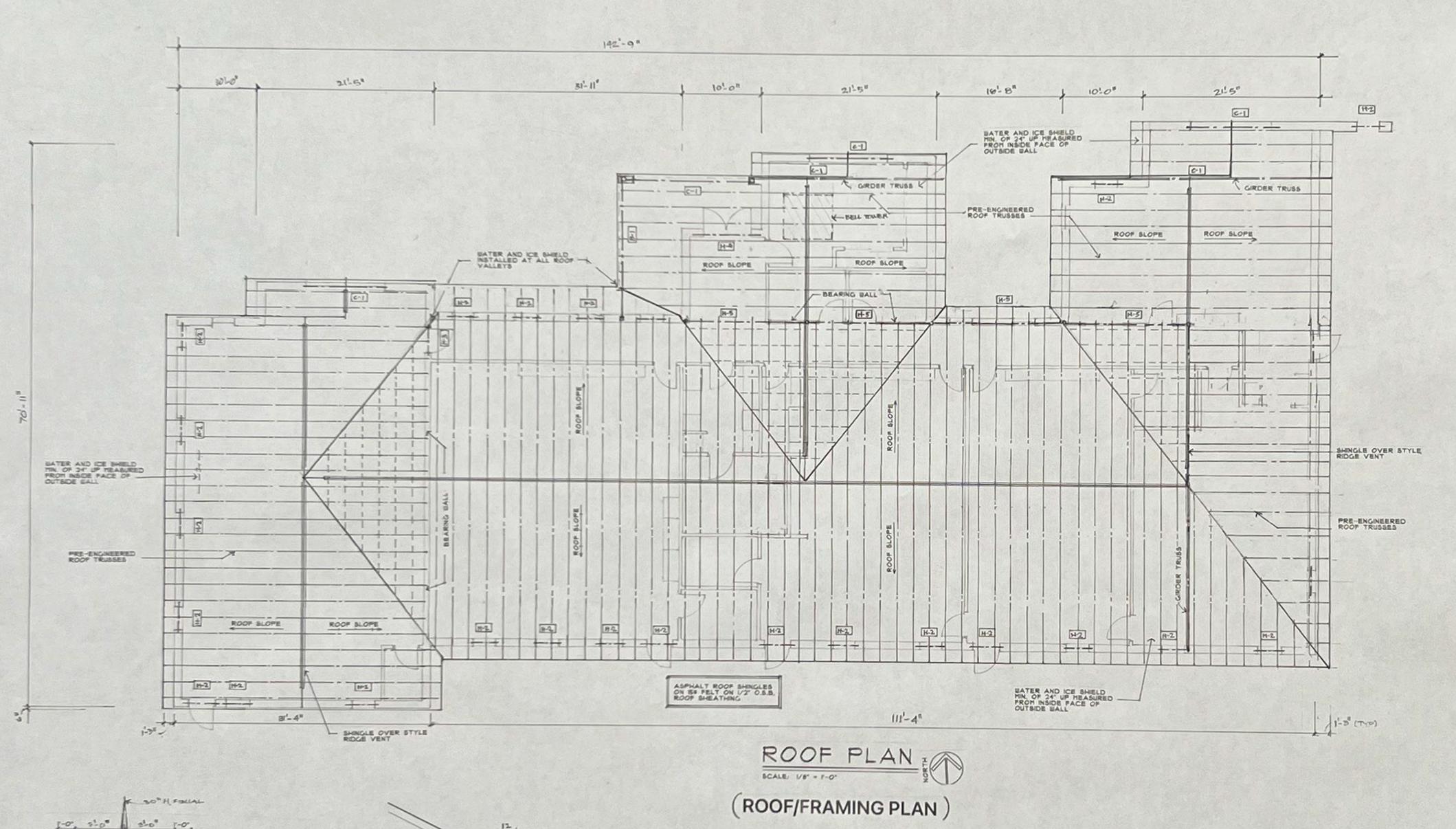
Scale AS NOTED

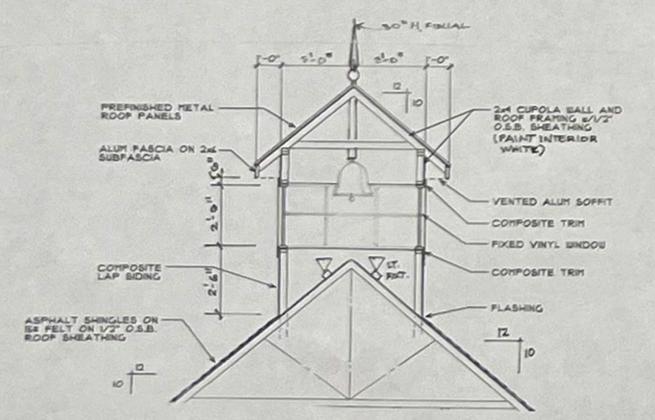
Project No. 24-010

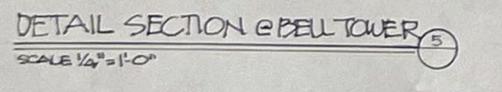
Sheet No.

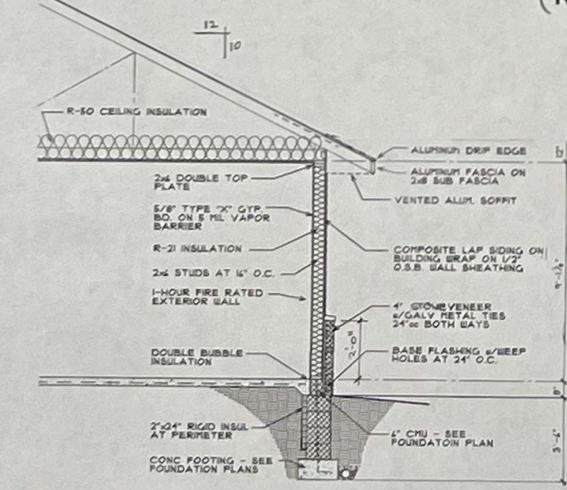
A-3

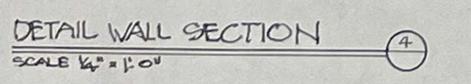


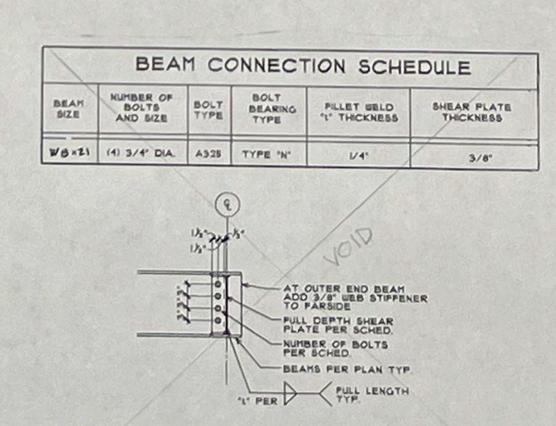




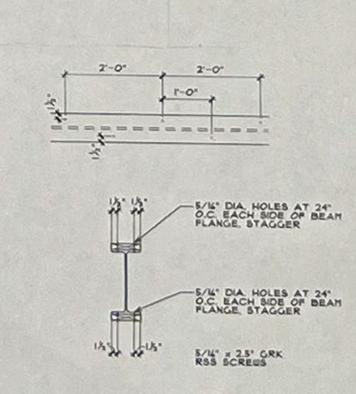














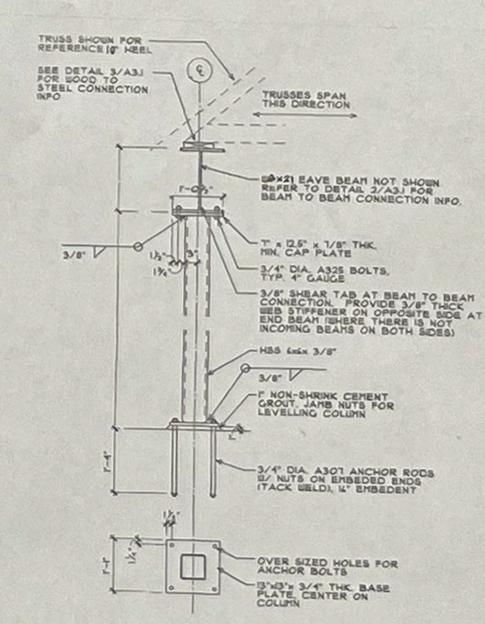
ROOF STRUCTURE NOTES

- REFER TO COVER SHEET FOR HINMUM GROUND SHOU LOADING FOR ROOF TRUSSES
- ROOF TRUSS DESIGN SHALL BE PERFORHED BY A REGISTERED ENGINEER IN THE STATE OF MICHGAN CONTRACTOR SHALL PROVIDE FINAL TRUSS SHOP DRAWING TO THE ARCHITECT AND THE BUILDING DEPARTHENT FOR REVIEW AND COMMENT, PLAN COPES SUBHITTED TO BUILDING DEPARTMENT SHALL BE SEALED BY A REGISTERED ENGINEER IN THE STATE OF
- CONTRACTOR SHALL PROVIDE ANCHORING AND BRACING FOR ALL ROOF TRUSSES AS REQUIRED BY ROOF TRUSS SHOP DRAWINGS
- ARCHITECT WILL PROVIDE REQUIRED ADDITIONAL TRUSS BRACING PLAN TO GENERAL CONTRACTOR AFTER SEALED TRUSS SHOP DRAWINGS ARE SUBHITTED TO ARCHITECT
- CONTRACTOR SHALL VERIFY BEIGHT AND LOCATION OF ALL ROOFTOP EQUIPMENT AND SUBHIT TO TRUSS ENGINEER FOR ADEQUATE REINFORCEMENT
- ALL ROOF TRUSSES SHALL BE CONNECTED TO WALL PLATES USING "SMPSON" H2.5 SEISHIC HURRICANE CLIPS AT EACH TRUSS CONNECTOR WITH TRUSS HANUFACTURER'S SHOP DRAWINGS
- 8. CONTRACTOR TO VERIFY ALL LOADING WITH TRUSS HANDFACTURER AND ARCHITECT PRIOR TO CONSTRUCTION
- I. STRUCTURAL LUMBER SUPPLIER SHALL SUBMIT SHOP DRAWING ON ALL STRUCTURAL LUMBER PRODUCTS INDICATING SIZE, GRADE AND DESIGN STRENGTH TO MEET CONDITIONS INDICATED IN HEADER AND BEAM SCHEDULE. SHOP DRAWING SHALL BE REVIEWED AND APPROVED BY ARCHITECT PRIOR TO CONSTRUCTION
- NO DRAFT STOPPING REQUIRED DUE TO SPRINKLER SYSTEM THROUGHOUT BUILDING INCLUDING ATTIC: PER DRAFTSTOPPING EXCEPTION IN MICHIGAN BUILDING CODE IF FIRE SUPPRESSION IS NOT SUPPLIED IN ATTIC. THEN DRAFTSTOPPING IS REQUIRED
- MATER AND ICE SHELD SHALL BE APPLIED A MINIMUM OF 24" MEASURED HORZ FROM INSIDE PACE OF OUTSIDE WALLS AT ALL ROOF OVERHANGS AND AT ALL ROOF VALLEYS PROVIDE INSULATED FIRE RATED ATTIC ACCESS DOORS WHEN IN FIRE RATED CEILING— SEE FLOOR PLAN FOR ATTIC ACCESS LOCATIONS. DOORS AND HARDWARE (HINGES, HOOK AND EYE) TO BE PROVIDE BY FRAMING CONTRACTOR
- 3. COORDINATE DOUBLE AND TRIPLE STUD FRAMING WITH PRE-ENGINEERED GROEF TRUSS LOCATIONS, REVIEW FINAL TRUSS PLANS FOR THESE LOCATIONS AND ANY OTHER ADDITIONAL LOCATIONS AS A RESULT OF TRUSS REVIEW

WOOD HEADERS AND BEAM SCHEDULE

- BEAR ON ONE () STUD.
- H-2 S-V4"x4-V2" 2.0E LVL, OR (2) 1-3/4"x4-V2" 2.0E LVL.
- H-3 5-1/4"-8-1/8" 2.0E LVL OR (3) 1-3/4"-1/8" 2.0E LVL BEAR ON THO (2) STUDS.
- H-4 5-V4"x8-1/8" 2.0E LVL BEAR ON THO (2) STUDS.
- H-S S-1/4"x8-1/8" 2.0E LVL BEAR ON THREE (3) STUDS.
- B- USXZI STEEL BEAM
- C- 5 x32x 5/4 LINTEL SEE DETAIL
- NOTE ALL HEADERS TO BE VERIFIED AFTER FINAL TRUSS SHOP DRAWINGS ARE AVAILABLE AND SUBMITTIED TO ARCHITECT

TRUSS PADRICATOR TO VEVIPY ALL ROOF SLOPE AS INDICATED ON DWGS NOTIFY ARCHITECT OF ANY ADJUGITHENTS TO THE PROPOSED ROOP SLOPES



COLUMN

Petikoski Architects

form | space | orde

41 Shoreham Grosse Pointe Shores, MI 482

tel: 313-930-2030 project

SCHOOL BELL CHILDCARE 4501 W. HIGHLAND ROAD MILFORD, MI 4838

owner

SCHOOL IN THE PINES INC

sheet

ROOF/FRAMING PLAN

The ideas and design concepts express herein and the graphically displayed arrangement of the components represented by this drawing have been destroyed for the exclusive use of the specified project and are the sein prperty of t architect. Any conveyence of disclosure the ideas or design concepts or use of a graphically displayed arrangements of th components shall be at the discretion of only through the expressed written cass of the Petionski Architects, L.L.C. @ PETROSIC ARCHITECTS, LL.C.

Issue / revision

Drawn by

JN/B(Checked by

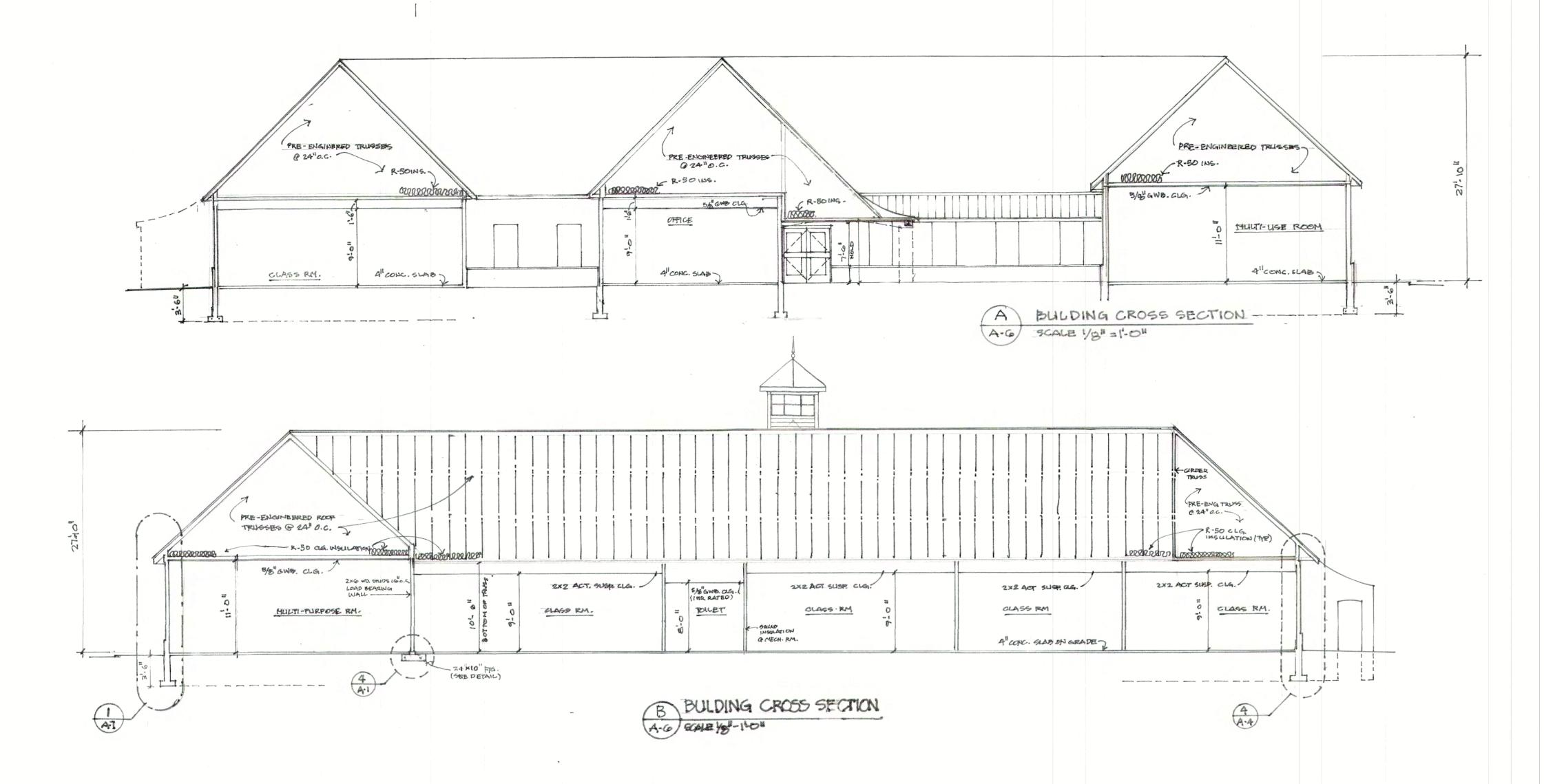
B-18-2024

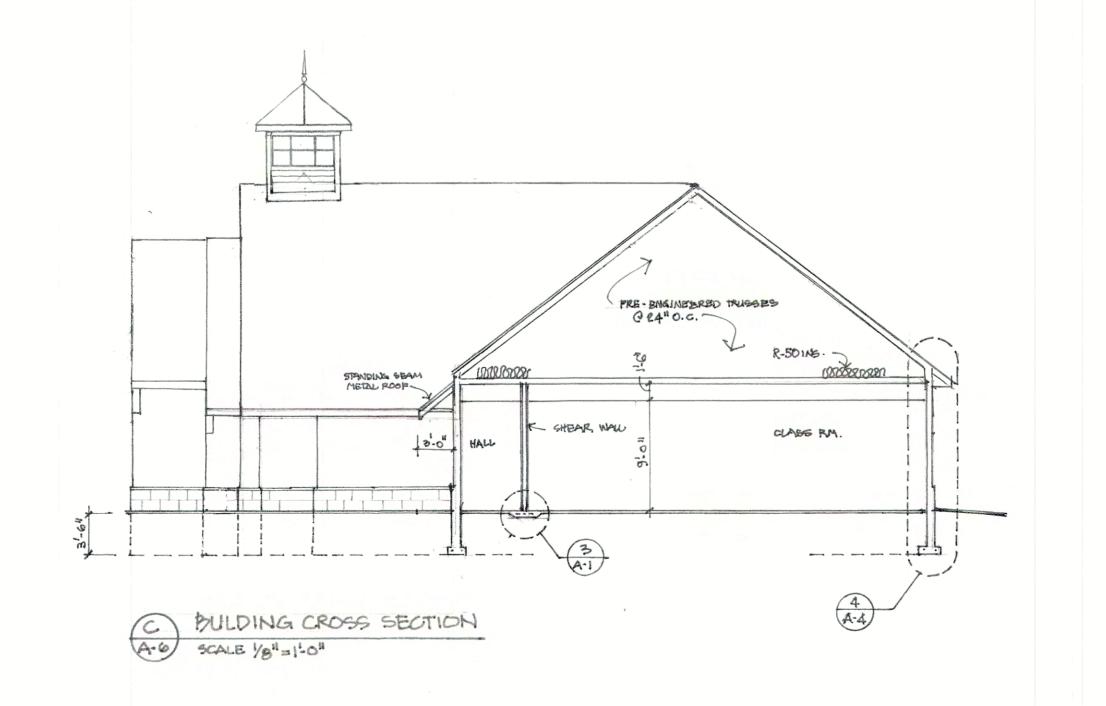
AS NOTEL

Project No. 24.010 Sheet No.

A-4







Petkoski Architects

form | space | order

41 Shoreham Grosse Pointe Shores, MI 48236 tel: 313-930-2030

project

SCHOOL BELL CHILDCARE 4501 W. HIGHLAND ROAD MILFORD, MI 48380

owner

SCHOOL IN THE PINES INC.

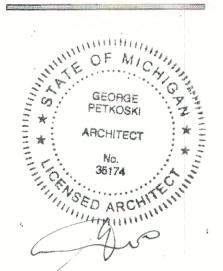
BUILDING **CROSS SECTIONS**

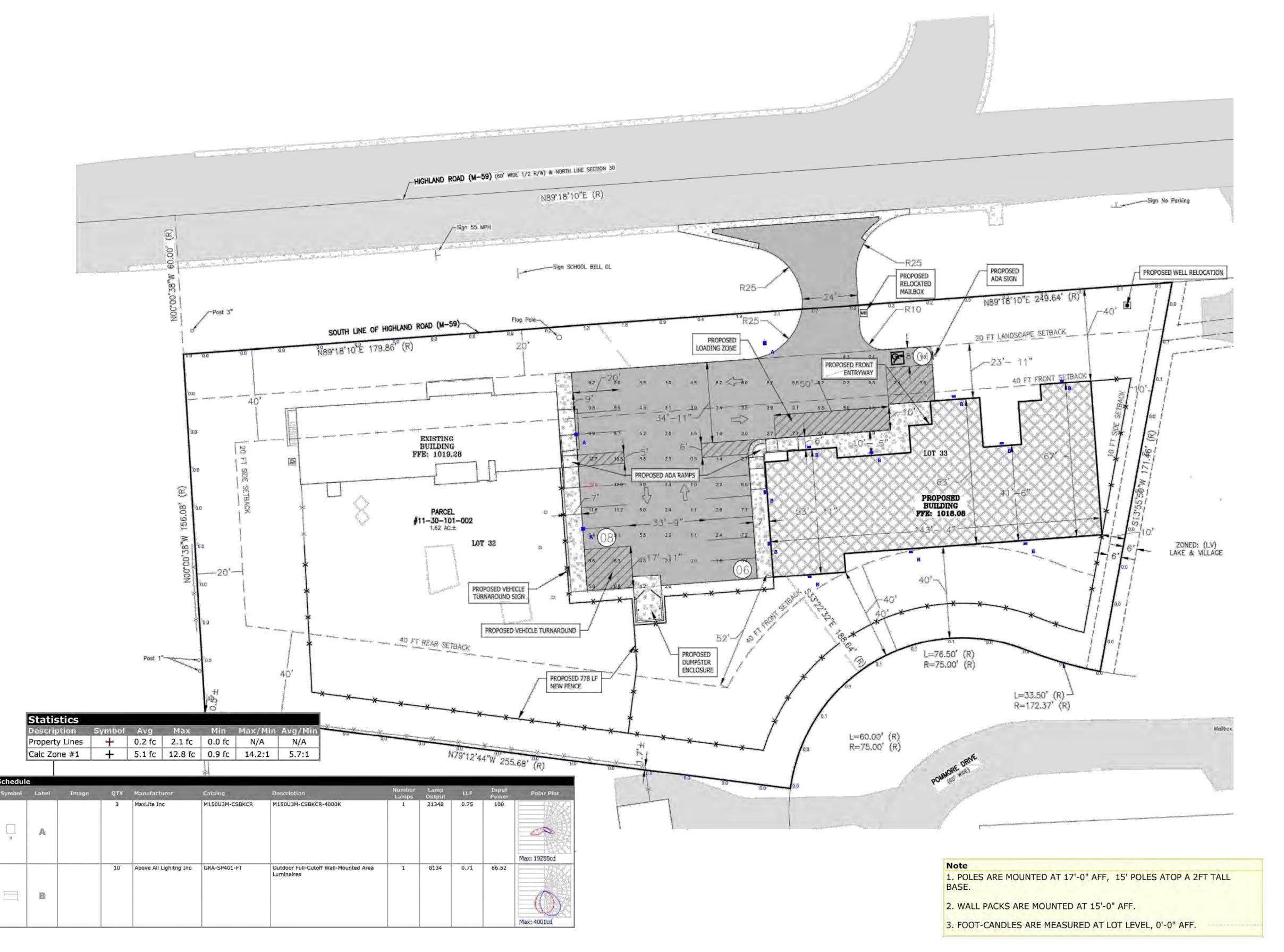
The ideas and design concepts expressed herein and the graphically displayed arrangement of the components represented by this drawing have been developed for the exclusive use of the specified project and are the sole prperty of the architect. Any conveyence of disclosure of the ideas or design concepts or use of any graphically displayed arrangements of their components shall be at the discretion of and only through the expressed written consent of the Petkoski Architects, L.L.C. issue / revision

Checked by

5-18-2024

AS NOTED







PHOTOMETRIC STUDY ON THIS SHEET SIMULATED TO CALCULATE LIGHT LEVELS ONLY. REFER TO SHEET E-01 FOR ALL FIXTURE LOCATIONS, SPECIFICATIONS, AND OTHER LIGHTING SYSTEM INFORMATION.

C.I. Engineering Solutions
15 N. WALNUT STREET, SUITE 300
MOUNT CLEMENS, MI 48043

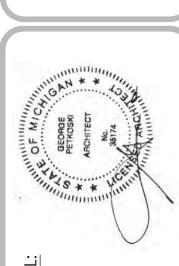
Engineering Solution

PLEASE DO NOT SCALE DRAWINGS
COPYRIGHT® 2019 CI ENGINEERING
SOLUTIONS, LLC.
THESE DRAWINGS ARE INTENDED FOR
USE BY THE CLIENT FOR THIS PROJECT
ONLY. ANY OTHER USE WITHOUT THE
DESIGNER'S WRITTEN PERMISSION IS
PROHIBITED.

SITE PHOTOMETRY

SCHOOL IN THE PINES INC.
4501 WEST HIGHLAND RD
MILFORD, MI 48380

PROJECT:
SCHOOL BELL DAYCARE



PROJECT NO.: MK-003



"Testing to keep America on a firm foundation"
4841 GOLF CLUB ROAD • HOWELL, MI 48843

May 08, 2024

Boss Engineering 3121 East Grand River Avenue Howell, MI 48843

Attention: Jared Prather

Reference: 4501 West Highland Road - Sub-Surface Exploration

Dear Mr. Prather,

Five (05) soil test borings designated as soil boring locations #1 through #5 were drilled in the influence of a planned new building located at 4501 West Highland Road in Highland Charter Township, Michigan. Soil test boring locations #1 and #5 were performed to determine the soil infiltration characteristics of the sub-surface sub-grade materials for a proposed underground detention system. Soil test borings #2 through #4 were performed in the influence of the proposed building to determine the structural integrity of the sub-surface sub-grade materials. Each of the test boring locations were advanced to a depth of twenty feet (20'-0") below the existing site grade. The soil boring locations can be identified on the enclosed diagram.

Soil descriptions and depths shown on the soil boring logs were approximate indications of changes from one soil to another and are not intended to represent an exact geological change or stratification.

Ground water was encountered in some of the test boring locations at the following depths:

Test boring #1 - None

Test boring #2 - None

Test boring #3 - 18'-6'' below the existing site grade.

Test boring #4 - 19'-0'' below the existing site grade.

Test boring #5 - 19'-0" below the existing site grade.

It should be noted that short-term ground water observations may not provide a reliable indication of the depth of the water table. In cohesive soils this is due to the slow rate of water infiltration into the bore hole as well as the potential for water to be trapped in overlying layers of granular soil in periods of heavy rain fall.

Information obtained from soil blow counts (standard penetration) indicate that the soils are variably compacted. The encountered granular soils were generally in a medium dense state, and the encountered cohesive soils generally had medium stiff to very stiff consistencies.



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

Soil blow counts, profiles and the unconfined compressive strengths of the encountered subgrade soils can be reviewed on the test boring logs.

Site Preparation

It is anticipated that there will be a minimal amount of fill material required for the site. Prior to the construction of footings or slabs all organic soils or unsuitable fill materials should be removed and it is recommended that the existing sub-grade be uniformly compacted to a minimum density of ninety-five percent of the materials maximum unit weight with a 20 ton vibratory roller. Engineered fill materials could then be placed in horizontal lifts not exceeding one foot in depth with each lift compacted uniformly to a minimum density of ninety-five percent of the materials maximum unit weight as determined by AASHTO T-180 or ASTM D1557. It is anticipated that the granular sub grade soils could be used as engineered fill materials in areas where an MDOT class III granular material is suitable. If any of the on-site cohesive material is reused as an engineered fill material it is recommended that the material be compacted with a padfoot (sheepsfoot) roller. Hastings Testing Engineers and Environmental Inc. should be on site during excavating and filling operations to verify the suitability of the native sub-grade and proposed engineered fill materials. It is recommended that all existing sub-grade soils be proof rolled with a fully loaded tandem axle dump truck or other approved equipment to determine if there is any sub-grade instability. Any unstable sub-grade materials should be removed and replaced with engineered fill material as described above.

Site Utilities

Temporary excavations for site utilities shallower than twelve feet should be able to use typical benched excavations at a 1.5:1 slope. For site utilities deeper than twelve feet it is recommended that a protective system be utilized such as trench boxes or shoring. The excavating contractor should be prepared for the potential of encountering perched ground water. Any ground water encountered at depths less than nineteen feet should be able to be removed with typical 2" pumps. Special dewatering techniques may be required for utilities placed deeper than the anticipated ground water elevation of approximately nineteen feet (19'-0) below the existing site grade. All temporary excavations should be performed according to OSHA standards and specifications.

The existing granular sub-grade materials could be suitable for backfilling utility trenches where a MDOT Class III granular material is required. The excavating contractor should be prepared to import material for utility trenches requiring MDOT class II granular material.



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

Foundation Recommendation

Based on the project information provided (one story building with slab on grade) and the results of field tests, it is believed that the proposed structure can be supported with conventional spread or strip footings. Prior to the construction of footings or floor slabs all organic soil and unsuitable fill materials should be removed and replaced with engineered fill materials as described in the site preparation portion of the report.

Footings could then be proportioned for a design soil pressure of two thousand pounds per square foot between one foot (1'-0") and ten feet (10'-0") below the existing site grade. Care should be taken to either remove or re-compact any loose granular or cohesive soils found in the bottom of the footing sub-grade locations. All exterior footings should be constructed at or below the maximum frost penetration of three feet six inches (3'-6") below finished grade. All footings and slabs should be constructed on naturally occurring sub-soils or engineered fill materials.

Floor Slab Recommendation

It is recommended that a minimum of six inches of clean coarse granular soil (MDOT class II granular material) be placed under all concrete slabs to provide a capillary break for any moisture migration. It is also recommended that a minimum 10 mil vapor barrier be installed directly under the concrete slab.

Expected Settlement

It is estimated that a properly configured shallow foundation constructed on the naturally occurring sub-grade soil or engineered fill material will have less than one inch of settlement and less than one-half inch of differential settlement.

Seismic Site Class

The known properties of the soils at the site are limited to the explored depths of the borings (20 feet) performed for this evaluation. Based on the conditions encountered in the borings, seismic site Class D applies to this site in accordance with the IBC and Michigan Building Code. The potential for liquefaction of the sub-grade soils should be considered low.

Detention Basin Infiltration

Test boring location #1 and #5 were drilled in the influence of the proposed underground detention system. Hastings Testing Engineers and Environmental Inc. was requested to perform permeability tests from soil sampled during drilling operations. The permeability tests were performed to determine the hydraulic conductivity of existing soils in the influence of the proposed detention pond.



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

Hastings Testing Engineers and Environmental Inc. performed the constant head/falling head permeability tests on five samples (ASTM D2434). The results of the tests are as follows:

Test #1

Sample: Soil boring location #1.

Depth: Approximately four feet below the existing site grade.

Sub-Grade Material: Coarse Brown Sand with Fine Gravel and some Silt

$$K_{sat}$$
= 3.8 $\frac{in}{hr}$

Test #2

Sample: Soil boring location #1.

Depth: Approximately nine feet below the existing site grade.

Sub-Grade Material: Silty Brown Clay

$$K_{sat} = 0.000088 \frac{in}{hr}$$

Test #3

Sample: Soil boring location #1.

Depth: Approximately nineteen feet below the existing site grade.

Sub-Grade Material: Coarse Brown Sand

$$K_{sat}$$
= 8.6 $\frac{in}{hr}$

Test #4

Sample: Soil boring location #5.

Depth: Approximately five feet below the existing site grade.

Sub-Grade Material: Coarse Brown Sand with Fine Gravel and some Silt

$$K_{sat}$$
= 2.0 $\frac{in}{hr}$

Test #5

Sample: Soil boring location #5.

Depth: Approximately seven feet below the existing site grade.

Sub-Grade Material: Silty Brown Clay

$$K_{sat} = 0.000031 \frac{in}{hr}$$



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

Conclusion

Experience indicates that the actual subsoil conditions at the site could vary from those generalized on the basis of the test borings made at specific locations. It is therefore essential that Hastings Testing Engineers and Environmental Inc. be notified of any variation of the soil conditions to determine the effects on the recommendations in this report. The evaluations and recommendations contained in the report have been formulated on assumed data relating to the proposed project. Any significant change in this data in the final design plans should be brought to our attention for review and evaluation.

If you should have further questions, please contact our office.

Sincerely,

Marc A. W. Smith PE

* MARC ANDREW WAYNE SMITH ENGINEER No. 6201068928



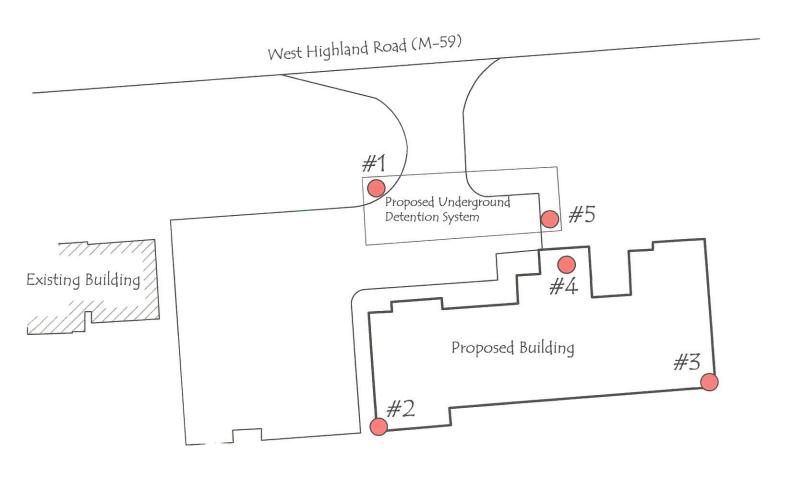
"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

4501 West Highland Road

Soil Boring Testing Diagram

Date: 04/26/2024







"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7029

3121 E. Grand River

CLIENT # :4562

PAGE

DATE :04/26/24 :1

Howell,

MI 48843

LOCATION : Soil Boring #1 - See Enclosed Diagram

	 Soil Description	Depth in	Sample &	0	N Val	% Water	Natural WT.		ed Strength
	-	Feet	Туре	*			PCF	Str. PSF	Fail Strain
	Organic Soil & Material	0.5							
	Coarse Brown Sand with Fine	1.0							
*	Gravel and some Silt	1.5		4					
 		2.0		6					
***		2.5	SS1	6	12	4.6			
***		3.0							
***		3.5		6					
***		4.0		7					
***		4.5	SS2	6	13	5.1			
***		5.0							
₩		5.5							
		6.0		5					
	Silty Brown Clay	6.5		9					
		7.0	SS3	13	22	10.8		9000.	
	 							Test - Dri	ving 2"

Type of Sample: SS - Split Spoon

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD SL - Split Spoon With Liner Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT GROUND WATER AFTER COMPLEATION: None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7029

3121 E. Grand River

CLIENT # :4562 DATE :04/26/24

PAGE :2

Howell,

MI 48843

LOCATION : Soil Boring #1 - See Enclosed Diagram

Soil	Description	Depth in	Sample &		N	% Water	Natural WT.	Unconfine	ed Strength
	Descripcion	Feet	Type	*			PCF	Str. PSF	Fail Strair
Silt	y Brown Clay	7.5							
		8.0							
		8.5		7					
		9.0		7					
		9.5	SS4	12	19	10.4		8000.	
H		10.0							
#		10.5							
#		11.0							
##		11.5							
#		12.0							
		12.5							
		13.0							
		13.5		9					
##		14.0		9					

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :None GROUND WATER AFTER COMPLEATION:None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7029

CLIENT # :4562

DATE :04/26/24

PAGE : 3

Howell,

MI 48843

LOCATION :Soil Boring #1 - See Enclosed Diagram

3121 E. Grand River

 Soil Description	Depth in	Sample		N Val	% Water	Natural WT.	Unconfine	ed Str	rength
	Feet	Type	*	Val		PCF	Str. PSF	Fail	Strain
Silty Brown Clay	14.5	 SS5 	10	19	11.3		7500.		
	15.0								
	15.5								
	16.0								
	16.5								
	17.0								
[17.5								
	18.0								
Coarse Brown Sand	18.5		6						
	19.0		9						
	19.5	SS6	12	21	5.0				
End Of Soil Boring #1	20.0								
Type of Sample: SS - Split Spoo SL - Split Spoo ST - Shelby Tuk	Liner	Samj	oler	18" w		Test - Dri Hammer, I			

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :None GROUND WATER AFTER COMPLEATION:None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7030

3121 E. Grand River

CLIENT # :4562

PAGE

DATE :04/26/24 :1

Howell, MI 48843

LOCATION : Soil Boring #2 - See Enclosed Diagram

	Soil Description	Depth in	Sample &	Count	N Val	% Water	1		ed Strength
		Feet	Type	*			PCF	Str. PSF	Fail Strain
	Organic Soil & Material	0.5							
*	Silty Brown Clay	1.0							
**** **** ****		1.5		4					
**** ****		2.0		6					
		2.5	SS1	7	13	14.6		2000.	
**************************************		3.0							
	Coarse Brown Sand with Fine	3.5		3					
	Gravel and some Silt	4.0		5					
開開		4.5	SS2	9	14	5.3			
		5.0							
開開		5.5							
88		6.0		6					
		6.5		9					
	Silty Brown Clay	7.0	SS3	9	18	15.5		2000.	
	Type of Sample: SS - Split Spoo							est - Driv	

ST - Shelby Tube

SL - Split Spoon With Liner Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :None

GROUND WATER AFTER COMPLEATION: None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7030

3121 E. Grand River

CLIENT # :4562

DATE : 04/26/24 PAGE

: 2

Howell,

MI 48843

LOCATION : Soil Boring #2 - See Enclosed Diagram

 Soil Description	Depth in	Sample &		N Val	% Water	Natural WT.	Unconfin	ed Strength
	Feet	Type	*			PCF	Str. PSF	Fail Strain
Silty Brown Clay	7.5							
	8.0					 		
LL Coarse Brown Sand	8.5		7					
ITT IJL IT IJL	9.0		8					
ini Iul Iul Iul	9.5	SS4	11	19	6.1			
而[LL	10.0							
	10.5							
ΠΓ LL ΠΓ	11.0							
	11.5							
	12.0							
	12.5							
	13.0							
	13.5		6					
	14.0		6					
- - - - - - - - - -								

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD :Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT

GROUND WATER AFTER COMPLEATION: None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7030

3121 E. Grand River

CLIENT # :4562

PAGE : 3

DATE : 04/26/24

Howell,

MI 48843

LOCATION : Soil Boring #2 - See Enclosed Diagram

Soil Description Coarse Brown Sand	in	Sample &	Count	N	% Water	Natural WT. P C F	Unconiii	ned Strengt
	Feet	Type	*				Str. PS	Fail Stra
rown Sand	14.5	SS5	8	14	5.9			
	15.0							
	15.5							
	16.0							
	16.5							
	17.0							
	17.5							
	18.0							
	18.5	<u> </u>	10					
	19.0		16					
	19.5	SS6	12	28	6.4			
oil Boring #2								
	ample: SS - Split	il Boring #2 19.5 ample: SS - Split Spoon	19.5 SS6 IBoring #2	il Boring #2 19.5 SS6 12	il Boring #2 19.5 SS6 12 28	il Boring #2	il Boring #2	il Boring #2

ST - Shelby Tube

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :None
GROUND WATER AFTER COMPLEATION:None



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7031

3121 E. Grand River

CLIENT # :4562

DATE : 04/26/24

PAGE

: 1

Howell,

MI 48843

LOCATION : Soil Boring #3 - See Enclosed Diagram

	Soil Description	Depth in Feet	Sample & Type		N Val	% Water	Natural WT. PCF		ed Strength Fail Strain
::::	Organic Soil & Material	0.5				8			
****	Coarse Brown Sand with Fine	1.0							
***	Gravel and some Silt	1.5		4					
		2.0		3					
388		2.5	SS1	4	7	5.6			
***		3.0							
***		3.5		4					
***		4.0		4					
**************************************		4.5	SS2	5	9	6.5			
***		5.0							
***		5.5							
8886	Silty Brown Clay	6.0		3					
		6.5		8					
IIII		7.0	SS3	6	14	9.9		9000.	

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :18' 6" BEG

GROUND WATER AFTER COMPLEATION:18' 6" BEG

GROUND WATER AFTER :1 Hour



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7031 CLIENT # :4562

3121 E. Grand River

DATE :04/26/24

PAGE

. 2

Howell,

MI 48843

LOCATION : Soil Boring #3 - See Enclosed Diagram

	Soil Description	Depth in	Sample &		N Val	% Water	Natural WT.	Unconfine	ed Strength
	SOII DESCRIPCION	Feet	Type	*			PCF	Str. PSF	Fail Strair
	Silty Brown Clay	7.5							
羅		8.0							
攤		8.5		8					
#		9.0		8					
腊		9.5	SS4	12	20	10.1		9000.	
開		10.0							
開開		10.5							
開開		11.0							
開開		11.5							
開開		12.0							
		12.5							
##		13.0		l l					
##		13.5		5					
開開	 	14.0		4					
	-								

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD :Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :18' 6" BEG GROUND WATER AFTER COMPLEATION:18' 6" BEG

GROUND WATER AFTER :1 Hour



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7031

3121 E. Grand River

CLIENT # :4562

DATE : 04/26/24 PAGE

: 3

Howell,

MI 48843

LOCATION : Soil Boring #3 - See Enclosed Diagram

	Soil Description	Depth in	Sample		N Val	 % Water	Natural WT.	Unconfine	ed Strength
		Feet	Type	*			PCF	Str. PSF	Fail Strain
	Silty Brown Clay	14.5	SS5	4	8	12.7		5000.	
		15.0							
#		16.0							
		16.5							
		17.0							
888		17.5							
		18.0							
	Coarse Brown Sand	18.5		3					
		19.0		3					
		19.5	SS6	3	6	8.9			
	End Of Soil Boring #3	20.0							
	Type of Sample. SS Split Spoo SL - Split Spoo		Liner						ving 2" OD Falling 30"

ST - Shelby Tube

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :18' 6" BEG

GROUND WATER AFTER COMPLEATION:18' 6" BEG



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7032

3121 E. Grand River

CLIENT # :4562 DATE :04/27/24

: 1

PAGE

Howell,

MI 48843

LOCATION : Soil Boring #4 - See Enclosed Diagram

Description anic Soil & Material	in Feet	&			TITALOW	WT.		
rganic Soil & Material		Type	* *	vai	Water	PCF	Str. PSF	Fail Strain
	0.5							
rse Brown Sand with Fine	1.0							
vel and some Silt	1.5		2					
	2.0		5					
	2.5	SS1	5	10	4.5			
	3.0							
	3.5		5					
	4.0		9					
	4.5	SS2	9	18	5.3			
	5.0							
	5.5							
ty Brown Clay	6.0		8					
	6.5		13					
	7.0	SS3	14	27	8.8	3	9000.	
t	cy Brown Clay	6.5	6.5	6.5 13	6.5 13	6.5 13	6.5 13	6.5 13 2000

Type of Sample: SS - Split Spoon

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD SL - Split Spoon With Liner Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG GROUND WATER AFTER COMPLEATION:19' 0" BEG



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7032

3121 E. Grand River

CLIENT # :4562

DATE :04/27/24 : 2 PAGE

Howell,

MI 48843

LOCATION : Soil Boring #4 - See Enclosed Diagram

	Depth	Sample &	Blow	N	% Water	Natural WT.	Unconfine	ed Strength
Soil Description	in Feet	Type	* *	vai	Water	PCF	Str. PSF	Fail Strair
Silty Brown Clay	7.5			 				
	8.0							
## ##	8.5		3					
	9.0		5					
	9.5	SS4	12	17	12.5		7000.	
	10.0							
**	10.5							
	11.0							
	11.5							
	12.0							
	12.5							
	13.0							
	13.5		2					
	14.0		4					

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN :T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG GROUND WATER AFTER COMPLEATION:19' 0" BEG



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7032

3121 E. Grand River

CLIENT # :4562

DAGE

DATE :04/27/24

PAGE

: 3

Howell,

MI 48843

LOCATION : Soil Boring #4 - See Enclosed Diagram

Soil Description	Depth	Sample	Blow	N Val	% Water	Natural WT.				
	Feel	Type	*			P C F	Str.	PSF	Fail	Strair
Coarse Brown Sand	14.5	SS5	12	16	6.0					
	15.0									
	15.5									
	16.0					<u> </u>				
	16.5								<u> </u>	
	17.0									
	17.5									
	18.0			<u> </u>	į		-			
	18.5		2		j I					
	19.0		4	<u> </u>	İ	<u> </u> 				
100 C	19.5	SS6	4	8	9.5					
End Of Soil Boring #4	20.0								<u> </u>	
										SALESSAN AND SALES

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG

GROUND WATER AFTER COMPLEATION:19' 0" BEG

GROUND WATER AFTER

:1 Hour



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

Howell,

REPORT # :7033

3121 E. Grand River

CLIENT # :4562

DATE :04/27/24 PAGE :1

MI 48843

LOCATION : Soil Boring #5 - See Enclosed Diagram

	Soil Description		Sample	e Blow N % Count Val Water		Natural WT.	Unconfined Strength			
		in Feet	& Type	* *	vai 	water	PCF	Str. PSF	Fail Strain	
	Organic Soil & Material	0.5								
		1.0								
***	Coarse Brown Sand with Fine	1.5		2						
***	Gravel and some Silt	2.0		2						
****		2.5	SS1	3	5	4.3				
***		3.0								
***		3.5		2						
888		4.0		4						
388		4.5	SS2	4	8	6.6				
388		5.0								
		5.5								
800	Silty Brown Clay	6.0		3						
		6.5		4						
		7.0	SS3	6	10	14.9		3000.		

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG GROUND WATER AFTER COMPLEATION: 19' 0" BEG



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7033

3121 E. Grand River

CLIENT # :4562

DATE :04/27/24 PAGE

: 2

Howell,

MI 48843

LOCATION : Soil Boring #5 - See Enclosed Diagram

	Soil Description	Depth in	Sample &		N Val	% Water	Natural WT.		ed Strength
		Feet	Type	*			PCF	Str. PSF	Fail Strain
	Silty Brown Clay	7.5							
		8.0							
		8.5		3					
I HIT		9.0		5					
		9.5	 SS4 	10	15	11.3		9000.	
		10.0							
		10.5							
		11.0							
		11.5							
		12.0							
		12.5				ļ 			
網票票票票票票票票票票票票票票票票票票	İ	13.0							
		13.5		5					
		14.0		4					
			İ						L

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30"

Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG

GROUND WATER AFTER COMPLEATION:19' 0" BEG

GROUND WATER AFTER :1 Hour



"Testing to keep America on a firm foundation"

4841 GOLF CLUB ROAD . HOWELL, MI 48843

REPORT OF SOIL BORING

TESTED FOR: Boss Engineering

REPORT # :7033

CLIENT # :4562

3121 E. Grand River

DATE :04/27/24

PAGE :3

Howell,

MI 48843

LOCATION : Soil Boring #5 - See Enclosed Diagram

Gail Degeniation	Depth in	Sample	Blow	N	% %	 Natural WT.	Unconfined Strength			
	Soil Description	Feet	∝ Type	*	vai	Water	PCF	Str. PSF	 Fail	Strain
	Coarse Brown Sand	14.5	SS5	10	14	6.7				
		15.0								
		15.5								
		16.0								
		16.5								
		17.0								
		17.5								
		18.0								
		18.5		2						
		19.0		4						
		19.5	SS6	10	14	10.7				
	 End Of Soil Boring #5	20.0								
									}	

Type of Sample: SS - Split Spoon

SL - Split Spoon With Liner

ST - Shelby Tube

* Standard Penetration Test - Driving 2" OD Sampler 18" with 140 # Hammer, Falling 30" Count made at 6" intervals.

DRILLING METHOD : Track Mounted Auger

DRILLING FOREMAN : T. Dingman

BACKFILL MATERIAL: Existing Material

GROUND WATER ENCOUNTERED AT :19' 0" BEG GROUND WATER AFTER COMPLEATION:19' 0" BEG

GROUND WATER AFTER

:1 Hour